

GENERAL NOTES:

A. CONSTRUCTION DOCUMENTS:

- 1. THE CONTRACTOR SHALL REVIEW THE APPROVED CONSTRUCTION DOCUMENTS AND NOTIFY THE ENGINEER OF ANY ERRORS OR DISCREPANCIES PRIOR TO THE START OF CONSTRUCTION.
2. CONTRACTOR IS RESPONSIBLE FOR USING QUALIFIED SUB CONTRACTORS EXPERIENCED IN THIS TYPE OF CONSTRUCTION.
3. THE CONTRACTOR SHALL FURNISH AND INSTALL EVERYTHING REQUIRED TO PROVIDE A COMPLETE STRUCTURE AS SHOWN HEREIN...

B. DIMENSIONS AND NOTATIONS:

- 1. WRITTEN DIMENSIONS SHALL TAKE PRECEDENCE OVER SCALED DIMENSIONS. DO NOT SCALE DRAWINGS.
2. FOR ANY MISSING DIMENSIONS REFER TO THE ARCHITECTURAL DRAWINGS. FOR ANY DIMENSIONS DISCREPANCIES USE DIMENSIONS FROM THE ARCHITECTURAL DRAWINGS.
3. ABBREVIATIONS USED ON THE APPROVED CONSTRUCTION DOCUMENTS SHALL BE CONSIDERED TYPICAL ABBREVIATIONS FOR THE INDUSTRY...

C. TYPICAL NOTES AND DETAILS:

- 1. SPECIFIC NOTES AND DETAILS SHALL TAKE PRECEDENCE OVER STANDARD TYPICAL NOTES AND DETAILS.
2. STANDARD TYPICAL NOTES AND DETAILS ARE TO BE USED WHEN REFERRED TO OR WHEN NO OTHER MORE RESTRICTIVE OR DIFFERENT DETAILS ARE SHOWN ON THE DRAWINGS.
3. WORK NOT PARTICULARLY SHOWN OR SPECIFIED SHALL BE THE SAME AS SIMILAR PARTS THAT ARE SHOWN OR SPECIFIED.

D. SHOP DRAWINGS:

- 1. SHOP DRAWINGS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER IN A TIMELY FASHION PRIOR TO FABRICATION AND CONSTRUCTION. UNLESS OTHERWISE STATED, A MINIMUM OF 5 WORKING DAYS AFTER RECEIPT OF SHOP DRAWINGS SHALL BE CONSIDERED AN ACCEPTABLE TIME PERIOD FOR THE STRUCTURAL ENGINEER REVIEW PROCESS.
2. A MINIMUM OF (2) HARD COPY SETS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW. THE STRUCTURAL ENGINEER WILL MAINTAIN (1) SET FOR REFERENCE PURPOSES. THE CONTRACTOR SHALL MAINTAIN (1) SET AT THE JOB SITE DURING THE DURATION OF CONSTRUCTION.

E. INSPECTIONS, SPECIAL INSPECTIONS, AND SITE VISITS (STRUCTURAL OBSERVATIONS):

- 1. INSPECTIONS BY THE BUILDING OFFICIAL ARE REQUIRED FOR CONSTRUCTION WORK FOR WHICH A PERMIT IS REQUIRED PER SECTION 109 OF THE IBC. CONTRACTOR IS REQUIRED TO COORDINATE AND SCHEDULE ALL REQUIRED INSPECTIONS WITH THE BUILDING OFFICIAL. INSPECTIONS PRESUMING TO GIVE AUTHORITY TO VIOLATE OR CANCEL PROVISIONS OF THE IBC OR OF OTHER ORDINANCES OF THE JURISDICTION SHALL NOT BE VALID.
2. SPECIAL INSPECTIONS ARE IN ADDITION TO, AND DO NOT REPLACE, THE INSPECTIONS BY THE BUILDING OFFICIAL PER CHAPTER 17 OF THE IBC. SPECIAL INSPECTIONS SHALL BE PERFORMED BY A QUALIFIED PERSON TO INSPECT AS REQUIRED ON THESE DOCUMENTS THE MATERIALS, INSTALLATION, FABRICATION, ERECTION OR PLACEMENT OF COMPONENTS AND CONNECTIONS REQUIRING SPECIAL EXPERTISE TO ENSURE COMPLIANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.

F. CODE REQUIREMENTS:

- ALL WORK SHALL CONFORM TO THE MINIMUM STANDARDS OF THE FOLLOWING CODES:
1. 2012 INTERNATIONAL BUILDING CODE (IBC)
2. ANY OTHER REGULATING AGENCIES WHICH MAY HAVE AUTHORITY OVER ANY PORTION OF THE WORK, INCLUDING THE STATE OF IDAHO.
3. SPECIFICATIONS, CODES AND STANDARDS NOTED SHALL BE OF THE LATEST APPROVED ISSUE, INCLUDING SUPPLEMENTS, UNLESS NOTED OTHERWISE.
4. CONTRACTOR SHALL BE PROPERLY REGISTERED IN THE STATE OF WASHINGTON PER WASHINGTON STATE LAW.
5. ALL STRUCTURAL MATERIAL MUST HAVE CURRENT ICC-ES REPORTS AVAILABLE UPON REQUEST TO PROVE CODE APPROVAL & INDUSTRY TOLERANCES

DESIGN CRITERIA:

A. 2012 INTERNATIONAL BUILDING CODE (IBC).

- 1. OCCUPANCY CATEGORY: II
2. NATURE OF OCCUPANCY: RESTAURANT
B. DESIGN LOADS:
1. ROOF:
a. LIVE LOAD = 25 PSF (SNOW)
b. DEAD LOAD = 18 PSF
2. PRE MANUFACTURED TRUSS- TOP CHORD:
a. LIVE LOAD = 25 PSF
b. DEAD LOAD = 10 PSF
c. WIND UPLIFT = 15 PSF
d. PARAPET PRESSURE = 52 PSF
3. PRE MANUFACTURED TRUSS- BOTTOM CHORD:
a. LIVE LOAD = 8 PSF
b. DEAD LOAD = 10 PSF
c. LIVE LOADS ARE NOT CONCURRENT
4. FLOOR- LIVE LOADS:
a. RESTAURANT = 100 PSF

C. IBC SEISMIC DESIGN:

- 1. SEISMIC DESIGN CATEGORY: B
2. IMPORTANCE FACTOR I\_e = 1.0
3. SOIL SITE CLASS: C
4. SEISMIC COEFFICIENTS:
S\_DS = 0.266
S\_D1 = 0.129
5. RESPONSE MODIFICATION: R= 6.5
SEISMIC FORCE RESISTING SYSTEM: BEARING WALL SYSTEM, LIGHT FRAMED WALLS WITH WOOD SHEAR PANELS
6. DESIGN BASE SHEAR:
V\_e = 0.029W
7. ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE
D. IBC WIND LOAD:
1. BASIC WIND SPEED = 115 MPH
2. EXPOSURE = C
3. IMPORTANCE FACTOR I\_w = 1.0
4. ANALYSIS METHOD= SIMPLE DIAPHRAGM
5. DESIGN BASE PRESSURE
P = 18 PSF

FOUNDATIONS:

A. MAXIMUM ALLOWABLE FOUNDATION SOIL BEARING PRESSURE:

- 1. 5000 PSF (DEAD + LIVE LOAD)
2. 6650 PSF (GRAVITY + LATERAL LOAD)
B. THE BOTTOM OF ALL EXTERIOR FOOTINGS SHALL BE 30 INCHES MINIMUM BELOW ADJACENT FINISHED GRADE.
C. THE INTERIOR FOOTINGS SHALL BE 12 INCHES MINIMUM BELOW FINISH FLOOR, U.N.O.
D. STRUCTURAL BACKFILL SHALL BE COMPACTED TO 95 PERCENT OF THE MAXIMUM DENSITY AS DETERMINED BY ASTM D1557. BRACE WALLS AND PIERS AS REQUIRED DURING BACKFILLING OPERATIONS.
E. PRIOR TO CONSTRUCTION, CONTRACTOR SHALL COORDINATE THE CONSTRUCTION DOCUMENTS, INCLUDING THE STRUCTURAL DRAWINGS, WITH THE GEOTECHNICAL REPORT. ANY DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE STRUCTURAL ENGINEER.
F. CONTRACTOR SHALL REFERENCE, AND MAINTAIN AT THE JOB SITE DURING CONSTRUCTION, THE GEOTECHNICAL REPORT PREPARED BY STRATA DATED APRIL 15, 2016.
G. DEFINITIONS:
1. STRUCTURAL WALLS - ANY LOAD BEARING WALL, SHEAR WALL, AND ANY WALL THAT REQUIRES A FOOTING.

CONCRETE:

A. REFERENCE STANDARDS:

- 1. ALL CONCRETE WORK SHALL CONFORM TO THE LATEST EDITION OF ACI 301
2. ALL CONCRETE SHALL BE NORMAL WEIGHT CONCRETE
3. CONCRETE MIX DESIGN SHALL BE ESTABLISHED IN ACCORDANCE WITH CHAPTER 5 OF ACI 318
4. USE LATEST EDITION OF ACI 308R WHEN CONCRETING DURING COLD WEATHER PER 19R.

- 1. SUPPLY PRODUCT DATA FOR PROPRIETARY MATERIALS AND ITEMS, INCLUDING REINFORCEMENT AND FORMING ACCESSORIES, ADMIXTURES, PATCHING COMPOUNDS, JOINT SYSTEMS, CURING COMPOUNDS AND OTHERS.
2. SHOP DRAWINGS FOR REINFORCEMENT DETAILING, FABRICATING, FOR BENDING, AND PLACING OF CONCRETE REINFORCEMENT SHALL COMPLY WITH ACI 315, MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES: BAR SCHEDULES, STIRRUP SPACING, BENT BAR DIAGRAMS, AND ARRANGEMENT OF CONCRETE REINFORCEMENT SHALL BE SHOWN. INCLUDE SPECIAL REINFORCING REQUIRED FOR OPENINGS THROUGH CONCRETE STRUCTURES.
C. FORMWORK AND FINISHES:
1. FORMWORK: DESIGN, ERECT, SUPPORT, BRACE AND MAINTAIN FORMWORK TO SUPPORT VERTICAL, LATERAL, STATIC AND DYNAMIC LOADS THAT MIGHT BE APPLIED UNTIL STRUCTURE CAN SUPPORT SUCH LOADS.
2. FINAL SLAB SURFACES SHALL RECEIVE A MACHINED STEEL TROWEL FINISH.
3. ANY PROJECTING CORNERS OF COLUMNS, BEAMS, WALLS, PEDESTALS, ETC SHALL BE FORMED WITH A 3/4 INCH CHAMFER.
4. DRY PACK, OR USE NON-SHRINK GROUT, UNDER BASE PLATES, BEARING PLATES, OR SILL PLATES AS REQUIRED FOR A LEVEL AND UNIFORM BEARING SURFACE. MINIMUM GROUT STRENGTH SHALL BE f\_c = 7000 PSI, U.N.O.
5. SEPARATE SLABS-ON-GRADE FROM VERTICAL SURFACES WITH JOINT FILLER.
D. MIX DESIGN, STRENGTH, AND ADMIXTURES:
1. 28-DAY COMPRESSIVE STRENGTHS (f\_c):
a. FOUNDATIONS = 3500 PSI
b. SLABS-ON-GRADE = 4000 PSI
2. CEMENT II OR III PER ASTM C-150
3. MAXIMUM SLUMP:
a. PRIOR TO ADDITION OF WATER-REDUCING ADMIXTURE = 4"
b. WITH ADDITION OF WATER-REDUCING ADMIXTURE= 10"
4. MAXIMUM SIZE COARSE AGGREGATE: 3/4 INCHES (PER ASTM C-33)
5. APPROVED ADMIXTURES:
a. FLYASH PER ASTM C-618
b. AIR ENTRAINING PER ASTM C-260
c. WATER REDUCING PER ASTM C-494

E. REINFORCEMENT:

- 1. REINFORCEMENT FOR CONCRETE:
a. ALL REINFORCING SHALL BE SUPPORTED IN FORMS SPACED WITH NECESSARY ACCESSORIES AND SHALL BE SECURELY WIRED TOGETHER IN ACCORDANCE WITH THE LATEST EDITION OF THE CRSI "MANUAL OF STANDARD PRACTICE"
b. DEFORMED BARS - ASTM A615, GRADE 60
c. WELDED WIRE REINFORCEMENT (WWR):
• SMOOTH WIRE - ASTM A185
• DEFORMED WIRE - ASTM A497
• USE FLAT MATS ONLY, NO ROLLED WWR IS PERMITTED.
2. MINIMUM REINFORCEMENT LAP = 40 BAR DIAMETERS
3. MINIMUM WWR LAP = GRID SPACING PLUS 2 INCHES
4. MINIMUM CONCRETE COVER OVER REINFORCEMENT:
a. CONCRETE CAST AGAINST EARTH = 3"
b. CONCRETE EXPOSED TO EARTH OR WEATHER = 1 1/2"
c. CONCRETE NOT EXPOSED TO EARTH OR WEATHER = 3/4"
5. SLAB-ON-GRADE REINFORCEMENT SHALL BE PLACED AT THE MID-DEPTH OF THE STRAND BOARD (OSB) AS LONG AS THE PANEL MEETS OR EXCEEDS THE CRITERIA LISTED BELOW.
3. ROOF SHEATHING SHALL BE, U.N.O.:
a. THICKNESS: 1/2"
b. SPAN RATING: 40/20
c. GRADE: PS-1/EXP 1
d. NAILING: PER PLANS
e. PLY CLIPS AT ALL UNSUPPORTED EDGES
f. MAXIMUM DISTANCE BETWEEN SUPPORT MEMBERS: 24"
4. WALL SHEATHING SHALL BE, U.N.O.:
a. THICKNESS: 1/2"
b. SPAN RATING: WALL-16
c. GRADE: PS-1/EXP 1
d. NAILING: PER SW SCHEDULE
e. BLOCKED AT ALL UNSUPPORTED EDGES
f. MAXIMUM DISTANCE BETWEEN SUPPORT MEMBERS: 16"
F. HEADERS:
1. PROVIDE ALL HEADERS AS SHOWN ON THE FRAMING PLANS. IF NO HEADER IS MARKED AT LOAD BEARING OR EXTERIOR WALL, PROVIDE HEADERS IN ACCORDANCE WITH THE FOLLOWING SCHEDULE:
OPENING WIDTH HEADER SIZE TRIMMER
UP TO 3'-0" WIDE TRIPLE 2x8 (1) 2x
3'-0" TO 4'-6" WIDE TRIPLE 2x10 (2) 2x
4'-6" TO 6'-6" WIDE TRIPLE 2x12 (2) 2x
2. PROVIDE THE MINIMUM STUD TRIMMER MEMBER IN THE SCHEDULE ABOVE UNDER ALL HEADERS, U.N.O. ON PLANS. PROVIDE A MINIMUM OF TWO FULL HEIGHT KING STUDS ON EACH SIDE OF ALL OPENINGS. BUILT-UP STUD COLUMNS SHALL BE NAILED TOGETHER WITH 16d NAILS AT 18" O.C. FOR THE FULL STUD HEIGHT. COMPARE HEADER STUDS WITH SHEAR WALL END STUDS - LARGER SIZE GOVERNS.
3. PROVIDE (3) 16d NAILS (OR LPT4) EACH END OF EACH HEADER TO CONNECT THE HEADER TO THE KING STUD (S). SEE ALSO NOTES 12 AND 14 UNDER TYPICAL LUMBER NAILING SCHEDULE.
4. ACCESSORIES AND FASTENERS:
1. ALL WOOD CONNECTORS SHALL BE SIMPSON STRONG-TIE OR APPROVED EQUAL AND INSTALLED PER MANUFACTURER'S RECOMMENDATIONS.
a. POST TO CONCRETE CONNECTIONS SHALL BE SIMPSON 'AB' POST BASES, U.N.O.
b. POST TO BEAM CONNECTIONS SHALL BE SIMPSON 'LPC2' POST CAPS, U.N.O.
c. SAWN LUMBER HANGERS SHALL BE SIMPSON 'LU' HANGERS, U.N.O.
2. NAILING SHALL BE IN ACCORDANCE WITH THE 2012 IBC TABLE 2304.9.1, UNLESS NOTED OTHERWISE.
3. NAILS SHALL BE COMMON WIRE NAILS (EXCEPT 16d NAILS MAY BE BOX WIRE NAILS).
4. METAL FINISH MATERIAL:
a. HIGH HUMIDITY AND PRESERVATIVE TREATED WOOD LOCATIONS: HOT DIPPED GALVANIZED STEEL PER ASTM A 153.
b. INTERIOR AND DRY LOCATIONS: STANDARD PAINTED OR ZINC GALVANIZED COATING.
H. DEFINITIONS:
1. APA RATED SHEATHING: A COMMON TRADE NAME THAT APPLIES TO A GRADE OR PANEL FOR USE AS SUBFLOORING, WALL SHEATHING, AND ROOF SHEATHING. PANELS ARE MADE WITH RESIN ADHESIVES THAT PROVIDE A MOISTURE RESISTANT BOND AND ARE DESIGNATED AS: EXPOSURE 1. PANELS CAN BE MANUFACTURED AS EITHER: PLYWOOD OR OSB.
2. APA STRUCTURAL 1 RATED SHEATHING: A SPECIAL SHEATHING GRADE DESIGNED FOR USE WHERE SHEAR AND/OR CROSS PANEL STRENGTH PROPERTIES ARE OF MAXIMUM IMPORTANCE. PANELS ARE MADE WITH RESIN ADHESIVES THAT PROVIDE A MOISTURE RESISTANT BOND AND ARE DESIGNATED AS: EXPOSURE 1. PANELS CAN BE MANUFACTURED AS EITHER: PLYWOOD OR OSB.
I. TYPICAL LUMBER NAILING SCHEDULE:
1. JOIST TO SILL OR GIRDER, TOENAILS: 3-8d
2. BRIDGING TO JOIST, TOE NAIL, EACH END: 2-8d
3. 1"x6" SUBFLOOR OR LESS TO EACH JOIST, FACE NAIL: 2-8d
4. WIDER THAN 1"x6" SUBFLOOR TO EACH JOIST, FACE NAIL: 2-16d
5. 2" SUBFLOOR TO JOIST OR GIRDER, BLIND AND FACE NAIL: 2-16d
6. SOLE PLATE TO JOIST OR BLOCKING, FACE NAIL: 16d @ 16" O.C.
7. TOP PLATE TO STUD, END NAIL: 2-16d
8. STUD TO SOLE PLATE: 2-16d END NAILS OR 4-8d TOE NAILS
9. DOUBLE STUDS, FACE NAIL: 16d @ 12" O.C.
10. DOUBLED TOP PLATES, FACE NAIL: 16d @ 16" O.C.
11. TOP PLATES, LAPS AND INTERSECTIONS, FACE NAIL: 2-16d
12. CONTINUOUS HEADER, TWO PIECES: 16d @ 16" O.C. ALONG EA. EDGE
13. CEILING JOISTS TO PLATE, TOE NAIL: 3-8d
14. CONTINUOUS HEADER TO STUD, TOE NAIL: 4-8d
15. CEILING JOISTS, LAPS OVER PARTITIONS, FACE NAIL: 3-16d
16. CEILING JOISTS TO PARALLEL RAFTERS, FACE NAIL: 3-16d
17. RAFTER TO PLATE, TOENAIL: 3-8d
18. 1" BRACE TO EACH STUD AND PLATE, FACE NAIL: 2-8d
19. 1"x8" SHEATHING OR LESS TO EACH BEARING, FACE NAIL: 2-8d
20. WIDER THAN 1"x8" SHEATHING TO EACH BEARING, FACE NAIL: 3-8d
21. BUILT-UP CORNER STUDS: 16d @ 24" O.C.
22. BUILT-UP GIRDER AND BEAMS 20d AT 32" O.C. AT TOP & BOTTOM: 2-20d AT ENDS AND @ EA. SPLICE
23. 2" PLANKS: 2-16d @ EACH BEARING
A. POST INSTALLED EXPANSION OR EPOXY ANCHORS SHALL BE PREAPPROVED BY THE STRUCTURAL ENGINEER PRIOR TO CONSTRUCTION UNLESS SPECIFICALLY DETAILED ON THE DRAWINGS.
B. HOLES MUST BE DRILLED AND CLEANED PER MANUFACTURER'S INSTRUCTIONS. ANCHORS MUST BE INSTALLED AND SPECIAL INSPECTED PER MANUFACTURER'S INSTRUCTIONS.
C. UNDER NO CIRCUMSTANCES WILL AN EXPANSION BOLT AND/OR EPOXY SYSTEM BE APPROVED WITHOUT A CURRENT ICC ES REPORT THAT MEETS THE REQUIREMENTS OF THE GOVERNING JURISDICTION AND IS IN ACCORDANCE WITH ACI 318 APPENDIX D AS ADOPTED BY THE IBC.

WOOD:

A. REFERENCE STANDARDS AND GOVERNING AGENCIES:

- 1. NDS FOR WOOD CONSTRUCTION
2. APA PANEL DESIGN SPECIFICATION
3. AWPA U1 - USE CATEGORY SYSTEM: USER SPECIFICATION FOR TREATED WOOD TRUSS CONSTRUCTION
4. TP1 NATIONAL DESIGN STANDARD FOR METAL PLATE CONNECTED WOOD TRUSS CONSTRUCTION
5. WMPA - WESTERN WOOD PRODUCTS ASSOCIATION
B. SUBMITTALS:
1. FABRICATED WOOD TRUSSES:
a. ALL ROOF TRUSSES SHALL BE DESIGNED, STAMPED, AND SIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF WASHINGTON.
b. TRUSS MANUFACTURER SHALL PROVIDE PROOF OF APPROVED THIRD PARTY INSPECTION AS REQUIRED BY THE 2012 IBC, SECTION 1704.2.
c. SUBMIT SHOP DRAWINGS OF PRE MANUFACTURED WOOD TRUSS LAYOUT FOR REVIEW BY THE ENGINEER PRIOR TO FABRICATION. TRUSS DESIGN DRAWINGS AND CALCULATIONS SHALL CONFORM TO THE REQUIREMENTS FROM SECTION 2303.4 OF THE IBC.
C. CARPENTRY
1. WOOD FRAMING MEMBERS SHALL HAVE THE FOLLOWING GRADES, OR BETTER, UNLESS NOTED OTHERWISE (U.N.O.):
a. BLOCKING: DOUGLAS FIR LARCH NO. 2, OR BETTER
b. BRIDGING: DOUGLAS FIR LARCH NO. 2, OR BETTER
c. STUD FRAMING: DOUGLAS FIR LARCH NO. 2, OR BETTER
d. BEAMS/HEADERS: DOUGLAS FIR LARCH NO. 2, OR BETTER
e. POSTS/BUILT-UP COLUMNS: DOUGLAS FIR LARCH NO. 2, OR BETTER
f. TOP AND BOTTOM PLATES: DOUGLAS FIR LARCH NO. 2, OR BETTER
2. MAXIMUM MOISTURE CONTENT OF ALL LUMBER AT THE TIME OF CLOSURE SHALL BE 19%
3. SPLICING OF WOOD MEMBERS, UNLESS SHOWN ON THE DRAWINGS, IS PROHIBITED WITHOUT WRITTEN APPROVAL OF THE PROJECT ENGINEER.
4. HOLES MAY BE DRILLED IN HEADER/BEAM IF SPECIFICALLY INDICATED ON THESE DRAWINGS. ANY OTHER HOLES OR NOTCHES ARE NOT ALLOWED.
5. ALL WOOD IN CONTACT WITH CONCRETE SHALL BE PRESSURE TREATED OR REDWOOD.
D. MANUFACTURED OR FABRICATED WOOD TRUSSES
1. ALL TRUSS LOADING SHALL SATISFY DEAD AND LIVE LOADS SHOWN UNDER DESIGN LOADS IN THE DESIGN CRITERIA, ABOVE.
2. MEMBER PROPERTIES: NO EXCEPTIONS OR SUBSTITUTIONS WITHOUT A WRITTEN REQUEST PRIOR TO FABRICATION.
a. CHORDS: DOUGLAS FIR LARCH NO. 2, OR BETTER
b. WEBS: DOUGLAS FIR LARCH NO. 2, OR BETTER
c. STUD, UTILITY, CONSTRUCTION, OR #3 GRADE WOOD IS NOT ACCEPTABLE FOR ANY TRUSS MEMBER
3. EACH TRUSS SHALL BE MARKED WITH THE FOLLOWING INFORMATION:
a. MANUFACTURER'S IDENTIFICATION
b. DESIGN LOAD(S)
c. TRUSS SPACING AND CONFIGURATION.
4. ALL TRUSS BLOCKING PANELS SHALL BE DESIGNED AND PROVIDED BY THE TRUSS MANUFACTURER AND CONSTRUCTED WITH APPROVED PLATES.
5. TRUSS PROFILES SHOWN ARE REPRESENTATIONS OF POSSIBLE CONFIGURATIONS OF WEB LOCATIONS, MEMBER SIZES, AND NUMBER OF PLYS. TRUSS MANUFACTURER SHALL VERIFY ALL TRUSS DIMENSIONS, ACCOUNTING FOR TOLERANCES, CONNECTIONS AND SPLICE REQUIREMENTS.
7. ALL TRUSS SPACING AND ORIENTATION DIRECTLY IMPACTS THE STRUCTURAL INTEGRITY OF THE FOUNDATION, AND WALL SYSTEM DESIGNS. ANY MODIFICATIONS TO THE TRUSS SPACING OR ORIENTATION MUST BE MADE IN WRITING AND SUBMITTED TO THE CONTRACTOR, AND ENGINEER PRIOR TO THE CONSTRUCTION OF THE ABOVE SYSTEMS.
8. THE TRUSS MANUFACTURER IS RESPONSIBLE FOR COORDINATION BETWEEN STRUCTURAL, ARCHITECTURAL, AND MECHANICAL LAYOUT REQUIREMENTS PRIOR TO FABRICATION.
E. PANEL SHEATHING:
1. STRUCTURAL WOOD SHEATHING AS SPECIFIED ON THESE DRAWINGS AT ROOF DIAPHRAGMS, SHEAR WALLS, AND BUILT-UP BLOCKING LOCATIONS SHALL BE STAMPED WITH THE SPECIFIED APA RATING.
2. STRUCTURAL WOOD SHEATHING MAY BE EITHER PLYWOOD OR ORIENTED

POST INSTALLED ANCHORS IN CONCRETE:

- A. POST INSTALLED EXPANSION OR EPOXY ANCHORS SHALL BE PREAPPROVED BY THE STRUCTURAL ENGINEER PRIOR TO CONSTRUCTION UNLESS SPECIFICALLY DETAILED ON THE DRAWINGS.
B. HOLES MUST BE DRILLED AND CLEANED PER MANUFACTURER'S INSTRUCTIONS. ANCHORS MUST BE INSTALLED AND SPECIAL INSPECTED PER MANUFACTURER'S INSTRUCTIONS.
C. UNDER NO CIRCUMSTANCES WILL AN EXPANSION BOLT AND/OR EPOXY SYSTEM BE APPROVED WITHOUT A CURRENT ICC ES REPORT THAT MEETS THE REQUIREMENTS OF THE GOVERNING JURISDICTION AND IS IN ACCORDANCE WITH ACI 318 APPENDIX D AS ADOPTED BY THE IBC.

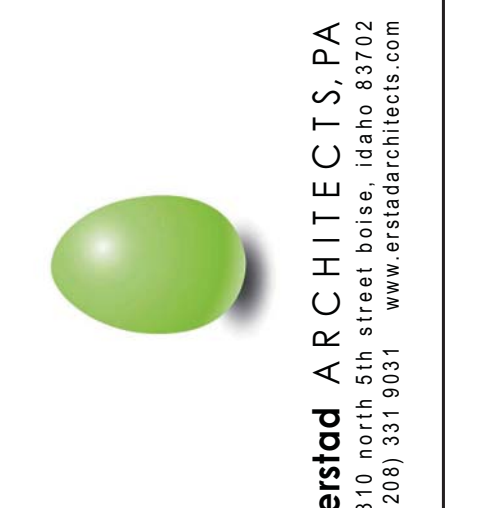
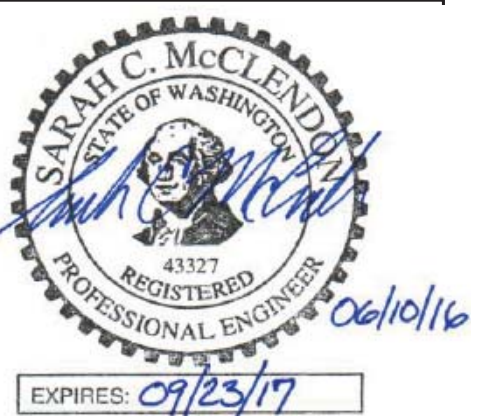
SPECIAL INSPECTION:

- A. SPECIAL INSPECTION AS HEREIN REQUIRED OF THE FOLLOWING MATERIALS, INSTALLATION, FABRICATION, ERECTION OR PLACEMENT OF COMPONENTS AND CONNECTIONS REQUIRING SPECIAL EXPERTISE TO ENSURE COMPLIANCE WITH APPROVED CONSTRUCTION DOCUMENTS AND REFERENCED STANDARDS.
B. STRUCTURAL OBSERVATION OF THE STRUCTURAL SYSTEM BY THE ENGINEER OF RECORD DOES NOT INCLUDE OR WAIVE THE RESPONSIBILITY FOR THE SPECIAL INSPECTION REQUIRED BY SECTION 109, 1704, OR OTHER SECTIONS OF THE INTERNATIONAL BUILDING CODE.
C. THE SPECIAL INSPECTION STATEMENT ON THIS SHEET LISTS THE ITEMS THAT REQUIRE SPECIAL INSPECTION AND VERIFICATION, THE CODE SECTION- REFERENCE FOR ADDITIONAL INFORMATION, AND THE REQUIRED FREQUENCY OF INSPECTION.

INSPECTION OF FABRICATORS: 1704.2
REQUIRED VERIFICATION & INSPECTION
FREQUENCY
1. WHERE FABRICATION OF STRUCTURAL LOAD-BEARING MEMBERS AND ASSEMBLIES IS BEING PERFORMED ON THE PREMISES OF A FABRICATOR'S SHOP, UNLESS THE FABRICATOR IS REGISTERED AND APPROVED TO PERFORM WITH SPECIAL INSPECTION. ONE TIME

SPECIAL CASES: 1704.15
INSPECTION OF MECHANICAL ANCHORS IN CONCRETE:
REQUIRED VERIFICATION & INSPECTION
FREQUENCY
1. THE SPECIAL INSPECTOR MUST BE ON THE JOB SITE CONTINUOUSLY DURING ANCHOR INSTALLATION TO VERIFY ANCHOR TYPE, ANCHOR DIMENSIONS, CONCRETE TYPE, CONCRETE INTEGRITY, HOLE DIMENSIONS, HOLE CLEANING PROCEDURES, ANCHOR SPACING, EDGE DISTANCES, CONCRETE THICKNESS, ANCHOR EMBEDMENT AND TIGHTENING TORQUE. PERIODIC

INSPECTION OF ADHESIVE ANCHORS IN CONCRETE:
REQUIRED VERIFICATION & INSPECTION
FREQUENCY
1. VERIFY HOLE DRILLING METHOD; HOLE LOCATION, DIAMETER AND DEPTH; HOLE CLEANING; ANCHORAGE ELEMENT TYPE, MATERIAL, DIAMETER AND LENGTH; ADHESIVE BRAND, TYPE AND EXPIRATION DATE; CONTINUOUS INSPECTION OF ADHESIVE MIXING AND INSTALLATION. CONTINUOUS



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PROTOTYPE T27-C-54 24508 - CONTINGUARY STAR
1230 N. Division Street, Spokane, WA 99202

PROJECT: 1089.15
DATE: 06/10/16
DRAWN: KM
CHECKED: SM

Permit Set

Table with 3 columns: No., DATE, DESCRIPTION. Contains 10 empty rows.

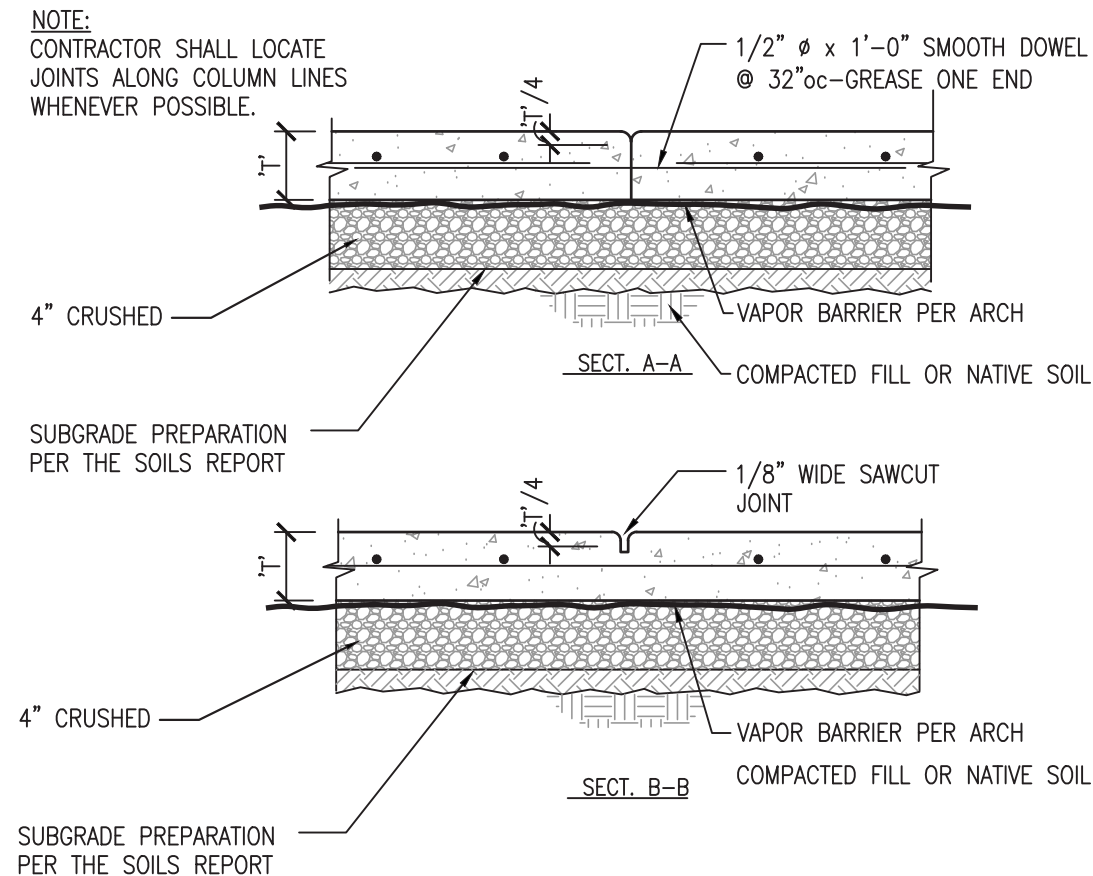
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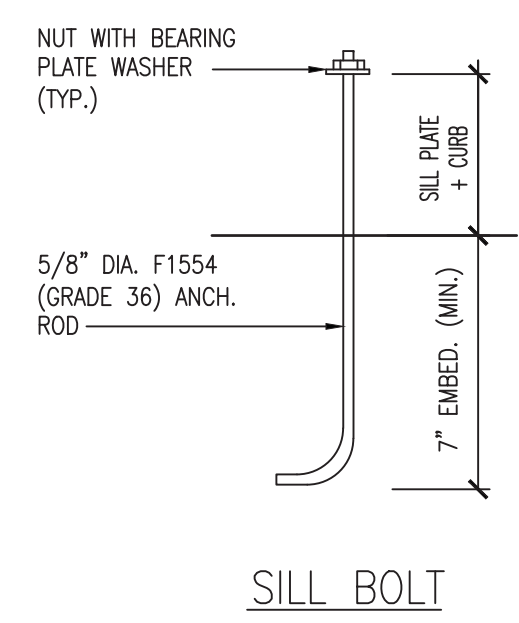
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SHEET INDEX
s0.0 GENERAL STRUCTURAL NOTES
s0.1 TYPICAL DETAILS
s1.0 FOUNDATION PLAN
S2.0 ROOF FRAMING PLAN
S3.0 FOUNDATION DETAILS
S4.0 ROOF FRAMING DETAILS

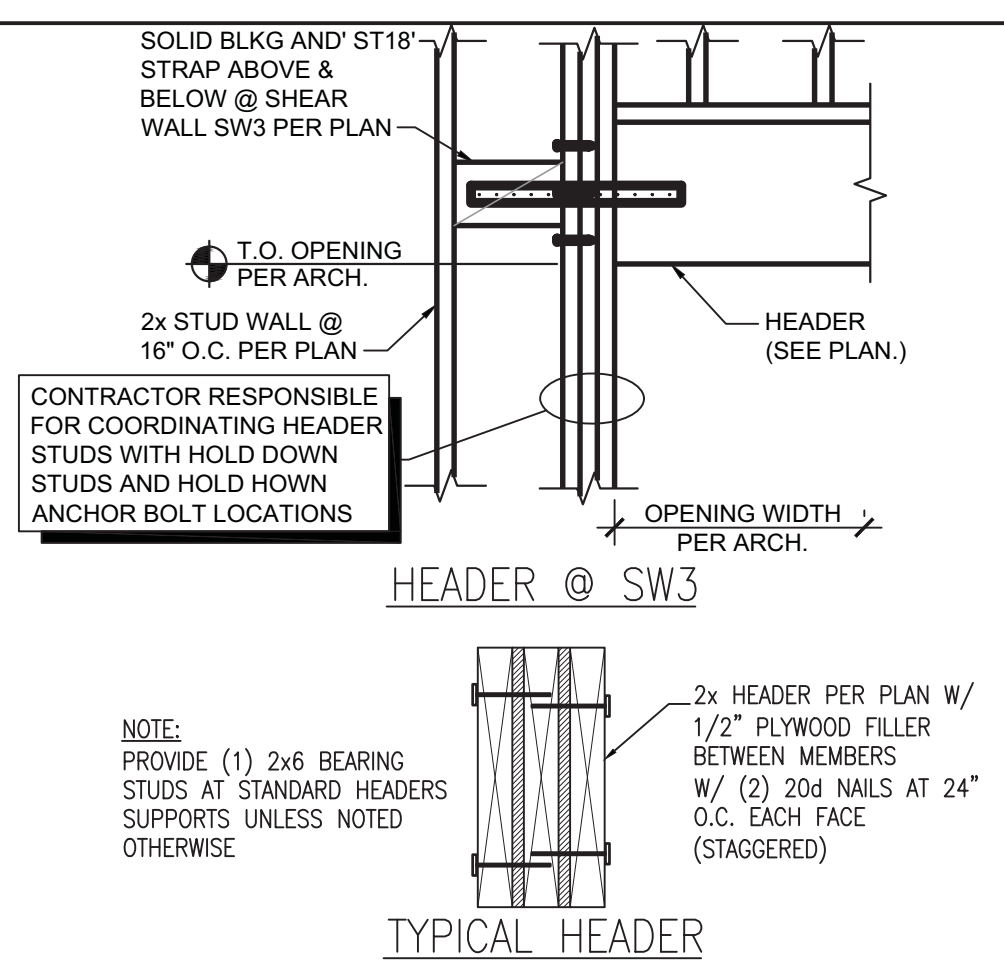




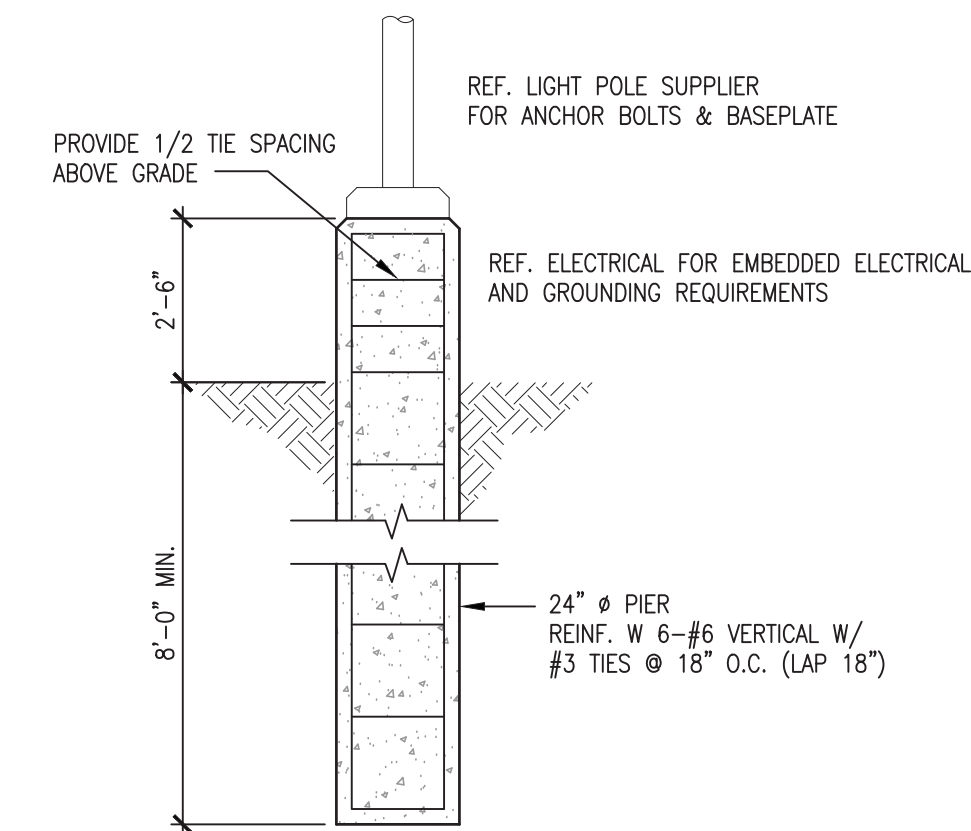
**CONSTRUCTION & CONTROL JOINT DETAILS**  
3/4" = 1'-0" **1**



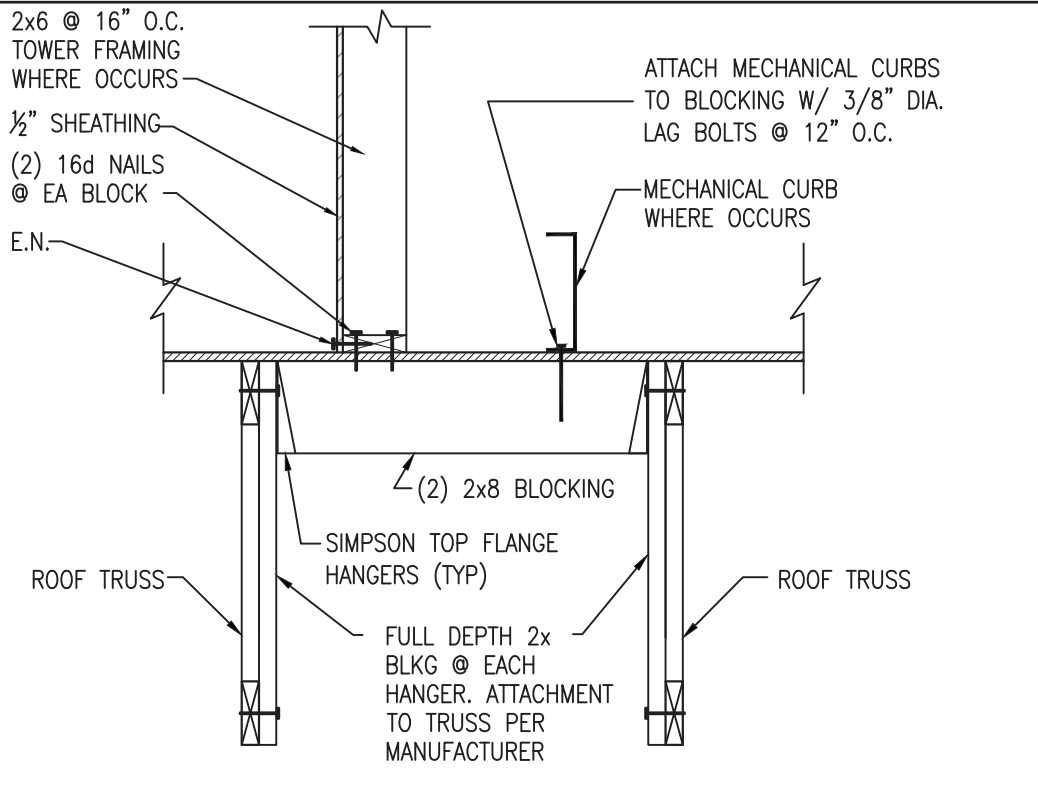
**TYPICAL SILL ANCHOR BOLT**  
1 1/2" = 1'-0" **2**



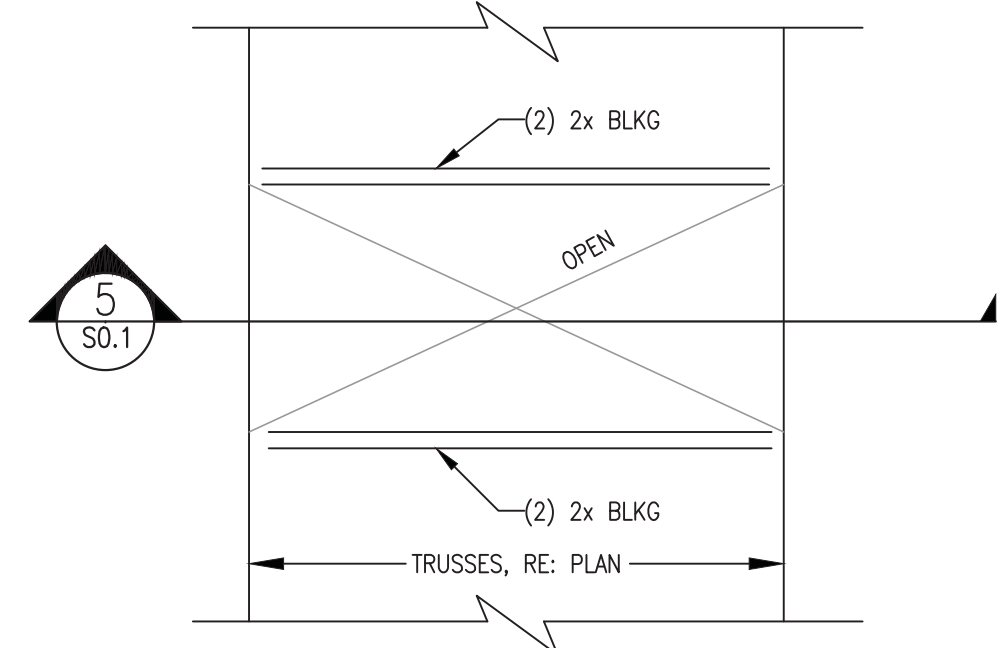
**HEADER DETAIL**  
1 1/2" = 1'-0" **3**



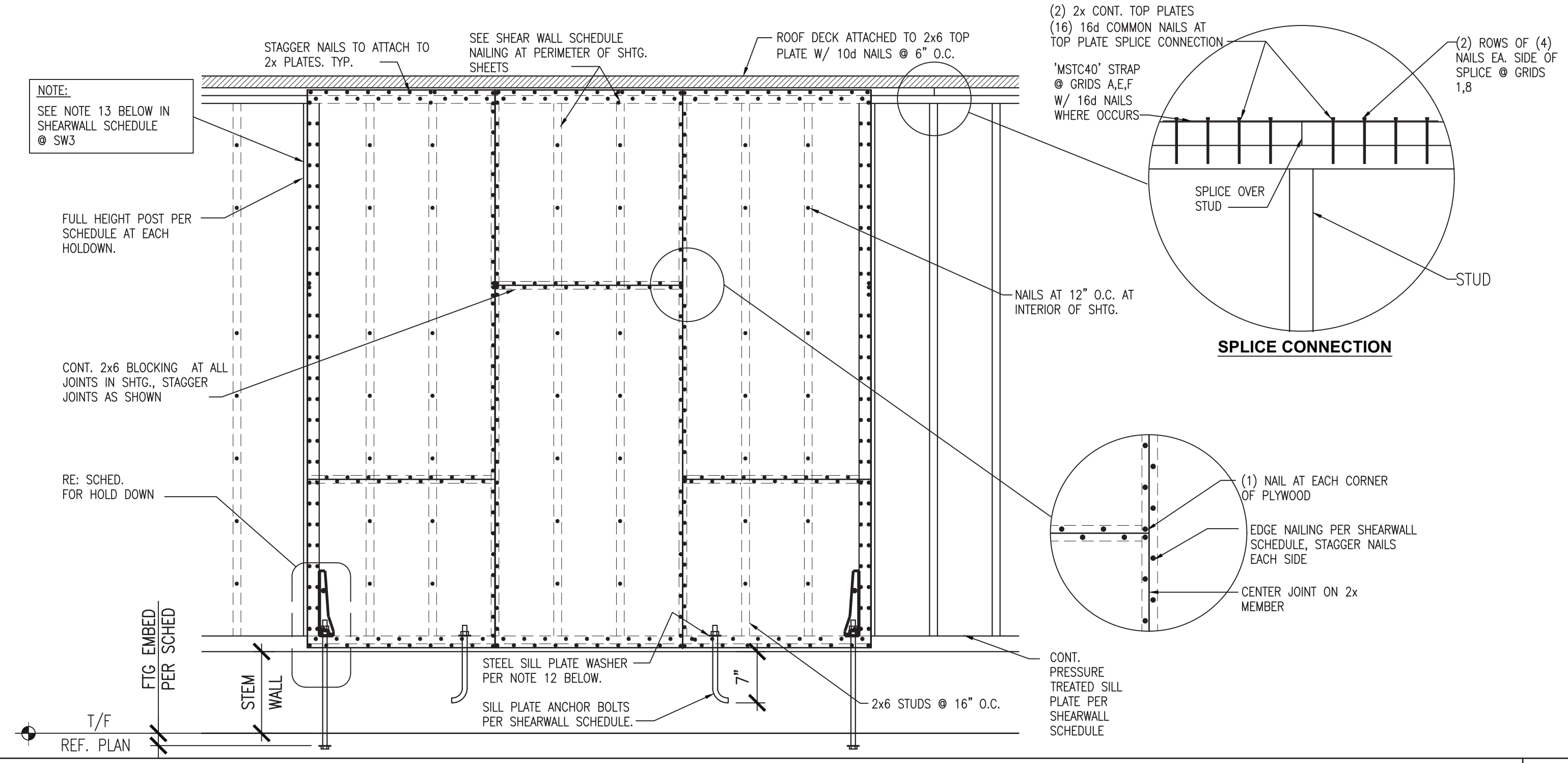
**LIGHT POLE BASE DETAIL**  
1/2" = 1'-0" **4**



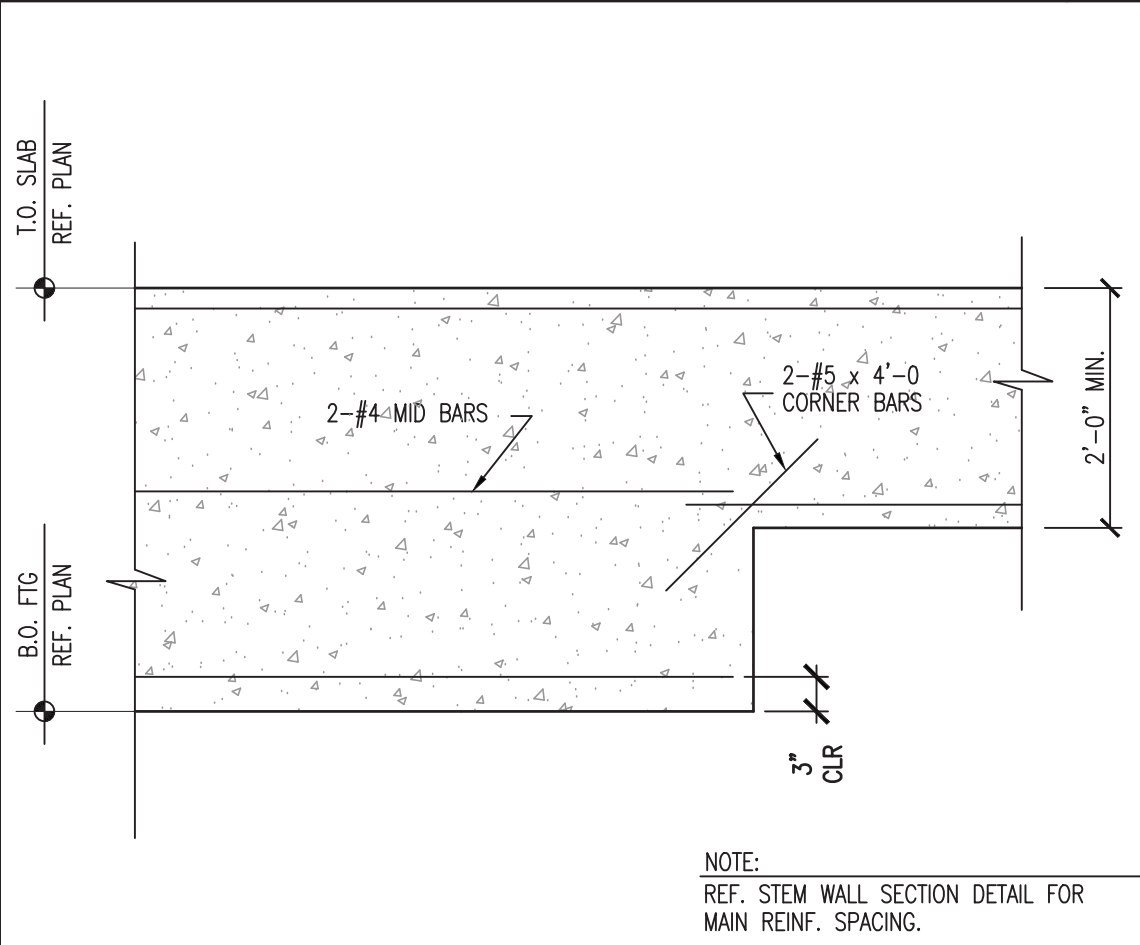
**SQUARE OR RECT. FRAME DETAIL**  
3/4" = 1'-0" **5**



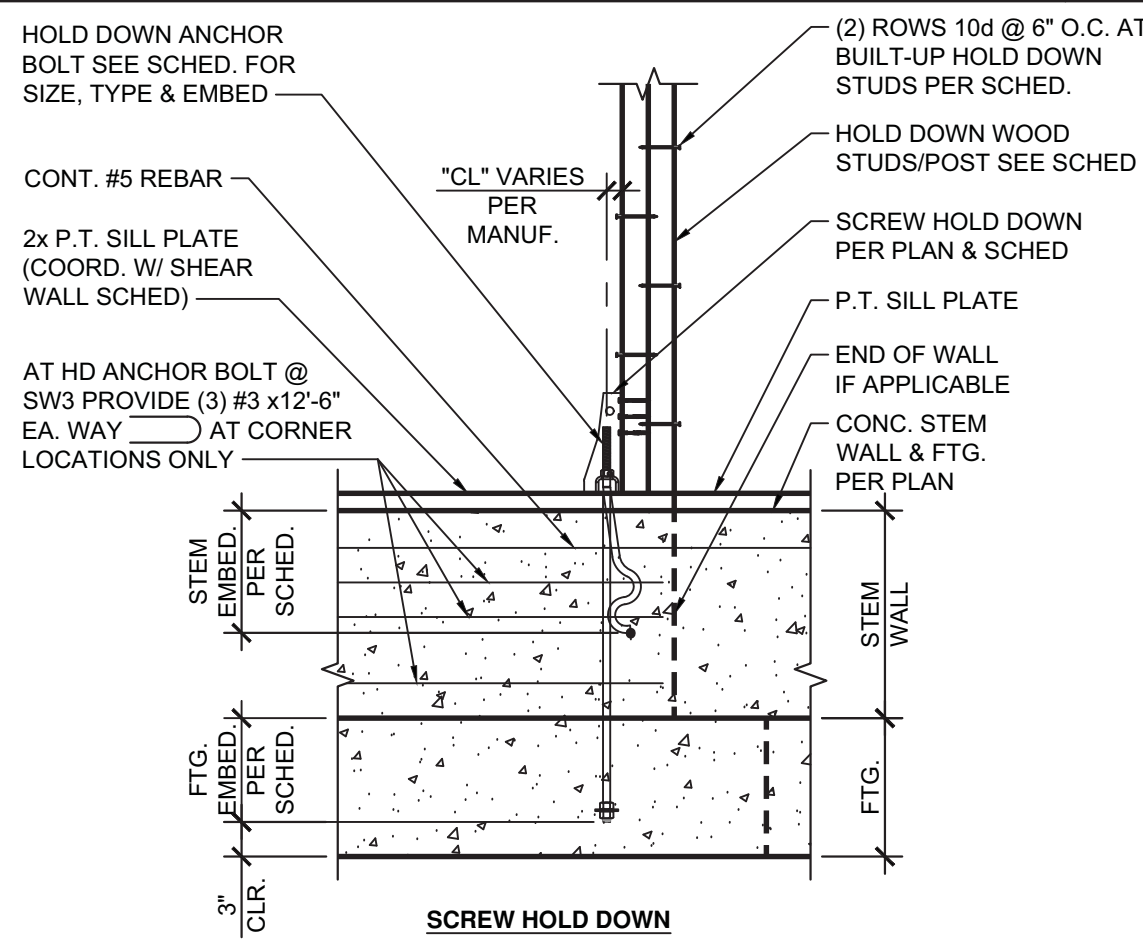
**SQUARE OR RECT. FRAME PLAN**  
1" = 1'-0" **6**



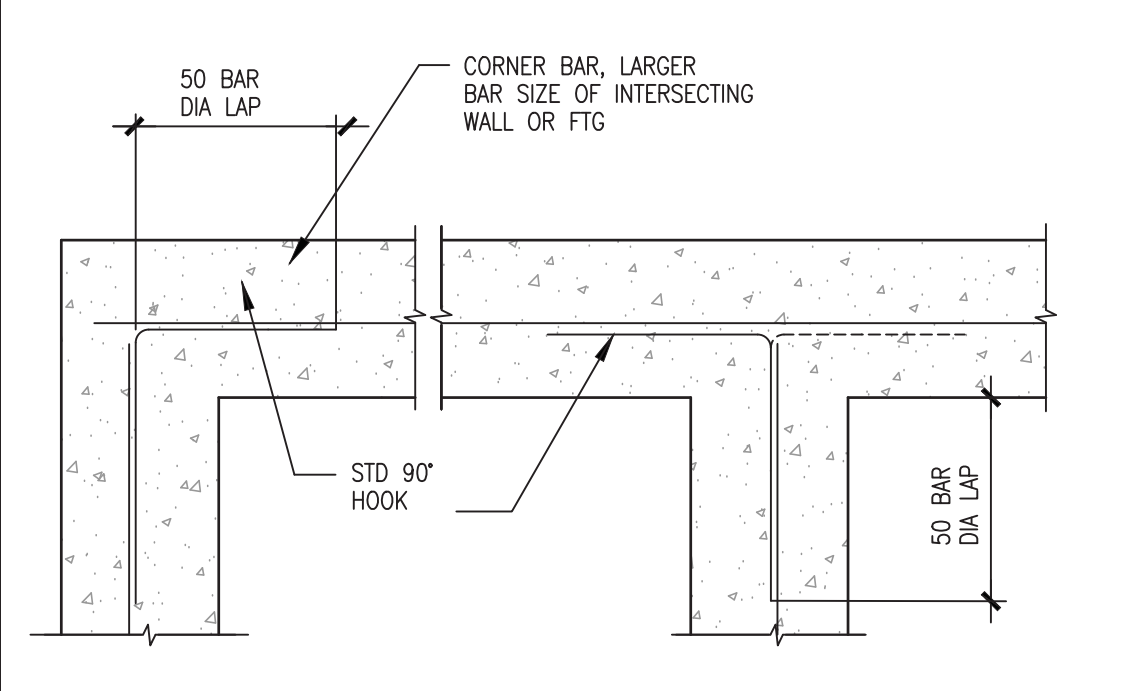
**SHEARWALL WALL ATTACHMENT**  
1" = 1'-0" **7**



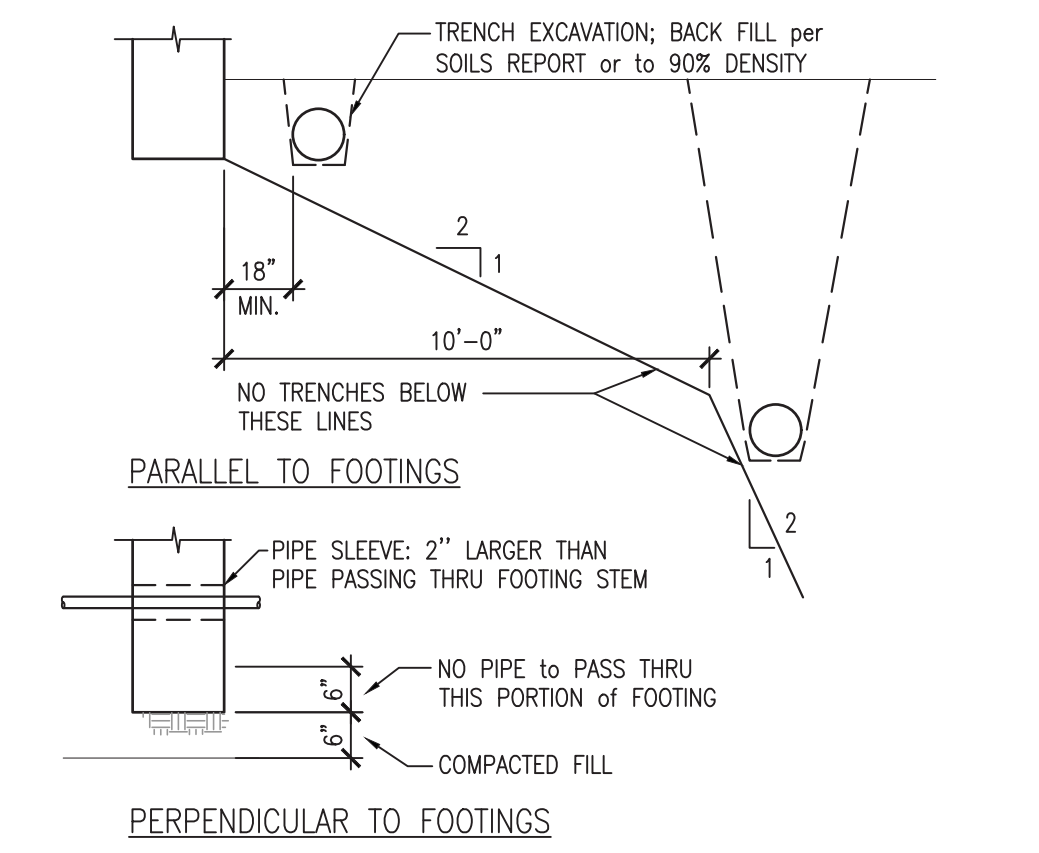
**TYPICAL STEM WALL STEP DETAIL**  
3/4" = 1'-0" **8**



**FOUNDATION HOLD DOWN DETAIL**  
3/4" = 1'-0" **9**



**TYPICAL STEM WALL CORNER BAR DETAIL**  
3/4" = 1'-0" **10**



**TYPICAL TRENCH DETAIL**  
NO SCALE **11**

MARK	PANEL GRADE	MIN. NOMINAL PANEL THICKNESS (N.)	MIN. NAIL PENETRATION IN FRAMING (N.)	NAIL SIZE	NAIL SPACING (N.)		SILL PLATE ANCHOR BOLTS	END STUDS/POST	HOLDDOWNS AT THE ENDS		BLOCKING CLIP	SHEAR WALL CAPACITY (PLF)
					PANEL EDGES	INTERMEDIATE FRAMING			TYPE	ANCHOR BOLT		
SW1	APA RATED STRUCTURAL SHEATHING	15/32	1 1/2	8d	6"	12"	2x SILL W/ 5/8" ø x 7" EMBED A.B. @ 48" O.C.	2x6 STUD	SIMPSON HDU2-SDS2.5	5/8" ø "STB24" W/ 21" EMBED (MIN) INTO STEM	'A35' @ 24" O.C.	365
SW2	APA RATED STRUCTURAL SHEATHING	15/32	1 1/2	8d	3"	12"	2x SILL W/ 5/8" ø x 7" EMBED A.B. @ 48" O.C.	4x6	SIMPSON HDU8-SDS2.5	7/8" ø "PAB7" W/ 6" FTG. EMBED	'A35' @ 16" O.C.	685
SW3	APA RATED STRUCTURAL SHEATHING	15/32	1 1/2	10d	2"	12"	2x SILL W/ 5/8" ø x 7" EMBED A.B. @ 32" O.C.	6x6	SIMPSON HDU14-SDS2.5	1" ø "PBA8" W/ 10" FTG. EMBED	'A35' @ 8" O.C.	1077

- NOTES:**
- REFER TO TYPICAL SHEARWALL ELEVATION, 7/SO.1.
  - ALL PANEL EDGES SHALL BE BACKED WITH 2 INCH NOMINAL OR WIDER FRAMING.
  - PANELS INSTALLED ON 2X STUDS SPACED 16 INCHES ON CENTER.
  - WHERE SILL PLATE ANCHOR BOLTS ARE MISSING OR MISLOCATED, USE SIMPSON "SET" EPOXY (ICBO ER-5279) OR EQUIVALENT WITH 5 INCHES MIN. EMBEDMENT WITH THREADED ROD OF THE SAME SIZE AND SPACING AS INDICATED. (THIS DOES NOT INCLUDE HOLD DOWN ANCHOR BOLTS)
  - IN ADDITION TO THE ON CENTER SPACING OF ANCHOR BOLTS, THERE SHALL BE AT LEAST (2) ANCHOR BOLTS PER PIECE OF SILL PLATE WITH A MINIMUM OF (1) ANCHOR BOLT LOCATED NOT GREATER THAN 9 INCHES, AND LESS THAN 4-1/2 INCHES OF EACH PIECE END.
  - MINIMUM CONCRETE SIDE EDGE DISTANCE FOR BOTH SILL PLATE AND HOLDDOWN ANCHOR BOLTS IS 2 INCHES AND MINIMUM CONCRETE END EDGE DISTANCE FOR HOLDDOWN ANCHOR BOLTS IS 12 INCHES.
  - HOLDDOWNS SHALL BE INSTALLED PER MANUFACTURER INSTALLATION REQUIREMENTS.
  - HOLES ARE NOT ALLOWED IN SHEARWALLS, UNLESS APPROVED BY THE STRUCTURAL ENGINEER PRIOR TO CONSTRUCTION. IF A HOLE IS APPROVED, IT IS TO BE ENTIRELY WITHIN ONE SHEET. PROVIDE BLOCKING AND EDGE NAILING AROUND OPENING.
  - MIN. WIDTH OF SHEATHING SHALL BE 2 FT.
  - NAILS SHALL BE COMMON NAILS.
  - OSB SHEATHING IS EQUAL TO PLYWOOD SHEATHING; 1/2" = 1/2" NOMINAL.
  - 0.229"x3"x3" IN SIZE STEEL SILL PLATE WASHER REQUIRED UNDER EACH NUT OF EACH ANCHOR BOLT FOR SW3
  - AT SW3: PANEL, PANEL NAILING, SILL PLATE BOLTS, AND BLOCKING CLIPS ARE CONT. FOR ENTIRE WALL LINE 1 ABOVE AND BELOW OPENINGS.



**erstad ARCHITECTS, PA**  
310 NORTH 5th Street, Idaho 83702  
(208) 331-9031 www.erstadarchitects.com

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PROJECT: 1089.15  
DATE: 06/10/16  
DRAWN: KM  
CHECKED: KH

**Permit Set**

SHEET TITLE:

**TYPICAL DETAILS**

SHEET NUMBER:

**SO.1**





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 (509) 331-9031 www.erstadarchitects.com

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**Conti's**  
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 1230 N. Division Street,  
 Spokane, WA 99202

PROJECT: 1089.15  
 DATE: 06/10/16  
 DRAWN: KH  
 CHECKED: SM

Permit Set

DATE	DESCRIPTION

SHEET TITLE:  
**FOUNDATION PLAN**

SHEET NUMBER:  
**S1.0**

**FOUNDATION PLAN NOTES:**

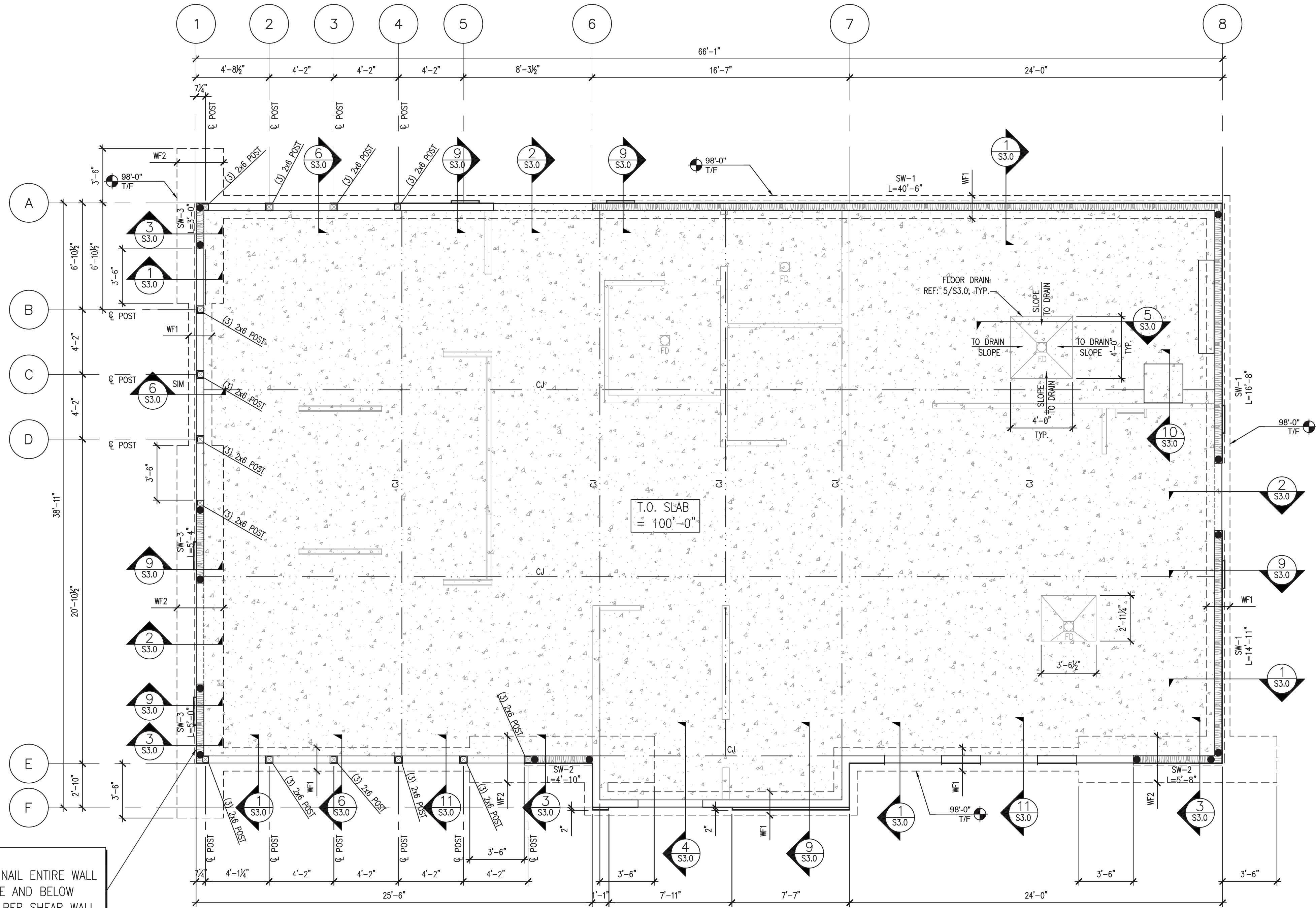
- REFER TO SHEET S0.1 FOR SHEARWALL SCHEDULES.
- REFER TO ARCHITECT'S KITCHEN AND PLUMBING DRAWINGS FOR ALL DRAIN SIZES AND LOCATIONS.
- REFER TO SHEET S0.0 FOR GENERAL NOTES AND SHEET S0.1 FOR TYPICAL DETAILS.
- T/F DENOTES TOP OF FOOTING ELEVATION = 98'-0".
- BOTTOM OF CONT. EXT. FTG. EL. = 97'-0" (TYP. U.N.O.)
- DENOTES HOLDDOWN LOCATION. HOLD ANCHOR TO BE EMBEDDED IN BOTH THE CONCRETE FOOTING AND STEM WALL. CONTRACTOR RESPONSIBLE FOR PROPERLY LOCATING & COORDINATING ANCHOR BOLT PLACEMENT. SEE SHEAR WALL SCHEDULE ON S0.1.
- CJ DENOTES CONTROL JOINT OR CONSTRUCTION JOINT. SEE S0.1.
- FOR ANY DIMENSIONS NOT SHOWN SEE ARCH PLANS.

**FOUNDATION PLAN LEGEND:**

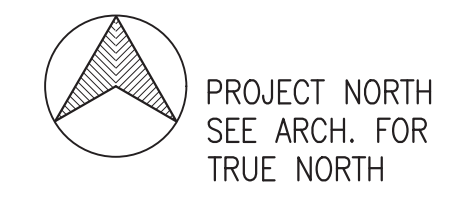
- INDICATES 2x6 @ 16" O.C. WOOD STUD WALL U.N.O.
- INDICATES STRUCTURAL WOOD SHEAR WALL. SEE SCHEDULE ON S0.1 DASHED LINE INDICATES WALL SHEATHING LOCATION AND DIMENSION INDICATES THE MINIMUM WALL SHEATHING LENGTH ± 2" PER 7/S0.1
- INDICATES WOOD POST, WITH 16d NAILS STAGGERED @ 24" O.C. AND WITH 'CB' POST BASE & 'BC' POST CAP, UNO.
- INDICATES 4" CONC. SLAB ON GRADE W/ #3 @ 12" O.C. EA. WAY (OR 6x6 W4.5xW4.5 WWR) (PLACED @ MID-DEPTH OF SLAB) OVER VAPOR BARRIER PER ARCH. OVER 4" COMPACTED 3/4" MINUS GRAVEL. SEE STRATA GEOTECH REPORT (FILE #SP16024A) DATED APRIL 15, 2016.
- INDICATES CONCRETE SLAB CONTROL JOINT. SEE 1/S0.1.

WALL FOOTING SCHEDULE			
FOOTING MARK	WIDTH (W)	DEPTH (D)	REINF.
WF1	1'-6"	12"	(2) #5 CONT. (L) W/ #5 @ 16" O.C. (T)
WF2	3'-0"	14"	(3) #5 CONT. (L) W/ #5 @ 16" O.C. (T)

- NOTES:**
- FOR ANY WALL FOOTING NOT MARKED, USE FOOTING TYPE WF1.
  - ALL FOOTINGS ARE CENTERED UNDER WALLS UNLESS NOTED OR DETAILED OTHERWISE.
  - (H) = HORIZONTAL BARS IN STEM WALL - WHERE OCCURS  
 (L) = LONGITUDINAL BARS IN FOOTING  
 (V) = VERTICAL BARS IN STEM WALL - WHERE OCCURS  
 (T) = TRANSVERSE BARS IN FOOTING  
 E.F. = EACH FACE  
 T&B = TOP AND BOTTOM  
 (V) VERTICAL BARS IN STEM WALL MAY BE BENT (IN ALTERNATE DIRECTIONS) @ THE FOOTING AND USED IN LIEU OF (T) TRANSVERSE BARS - SEE DETAILS.



NOTE:  
 SHEATH & NAIL ENTIRE WALL LINE (ABOVE AND BELOW OPENINGS) PER SHEAR WALL TYPE SW3 @ WALL LINE ①







**erstad ARCHITECTS, PA**  
 310 NORTH 5th Street, Suite 83712  
 (206) 331-9031 www.erstadarchitects.com



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PROJECT: 1089.15  
 DATE: 06/10/16  
 DRAWN: KH  
 CHECKED: SM

Permit Set

DATE	DESCRIPTION

SHEET TITLE:  
**ROOF FRAMING PLAN**

SHEET NUMBER:

**S2.0**

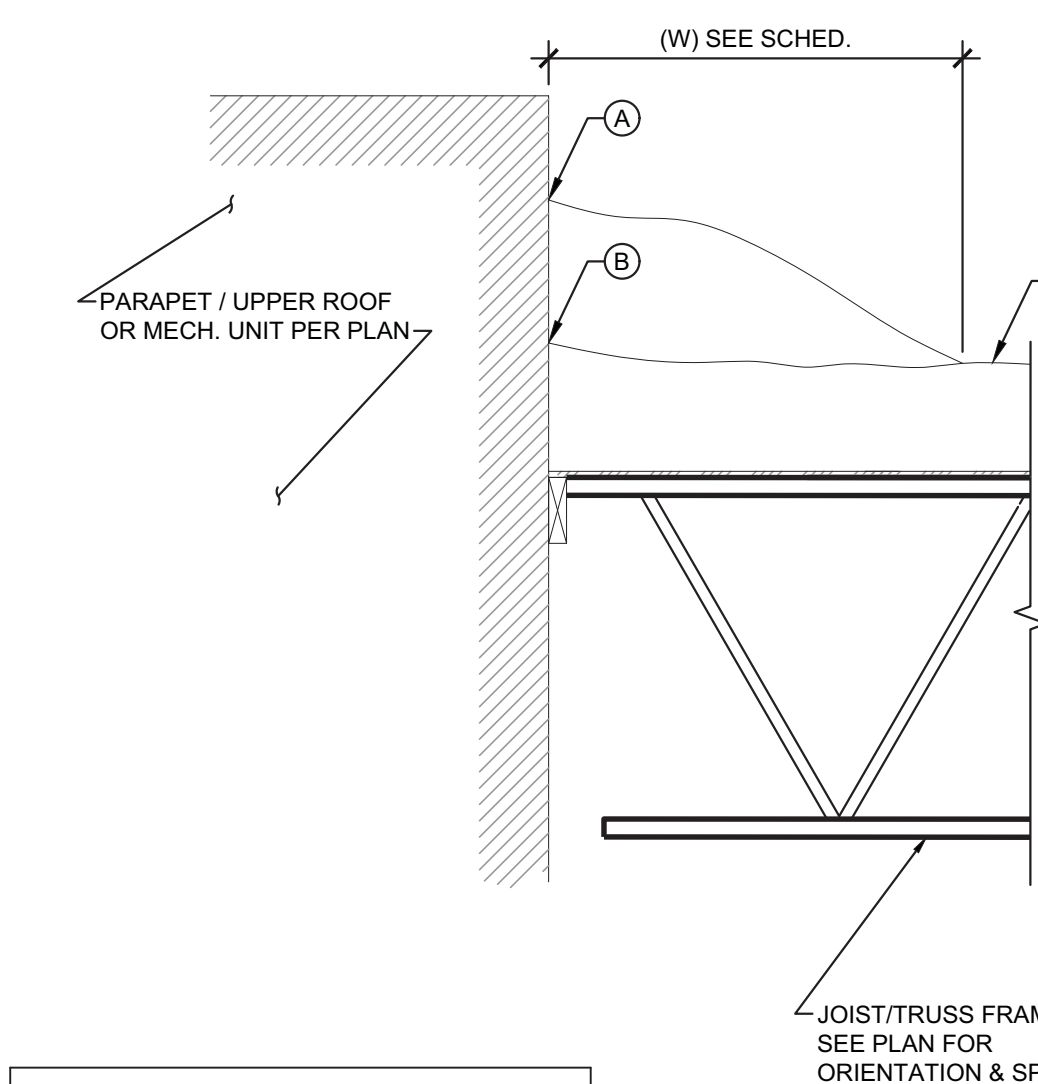
**PLAN NOTES:**

- INDICATES SHEAR WALLS BELOW ROOF.
- INDICATES HEADER PER GENERAL STRUCTURAL NOTES S0.0 (U.N.O.)
- INDICATES 6" WALL ABOVE ROOF.
- REFER TO 6/S0.1 FOR FRAMING AT ROOF OPENINGS.
- INDICATES PARAPET BRACES, RE: SHEET S4.0 FOR DETAILS.
- REFER TO SHEET S0.0 FOR GENERAL NOTES.
- REFER TO 8/S4.0 FOR ROD HANGER SUPPORT DETAIL.
- TRUSS BEARING ELEVATION = 112'-10".
- ROOF SHEATHING: 19/32" APA RATED SHGT W/ 10d @ 6" O.C. E.N. AND 10d @ 12" O.C. F.N.

**ROOF FRAMING PLAN KEYNOTES:**

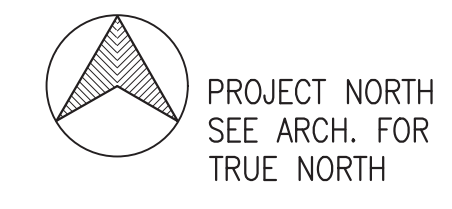
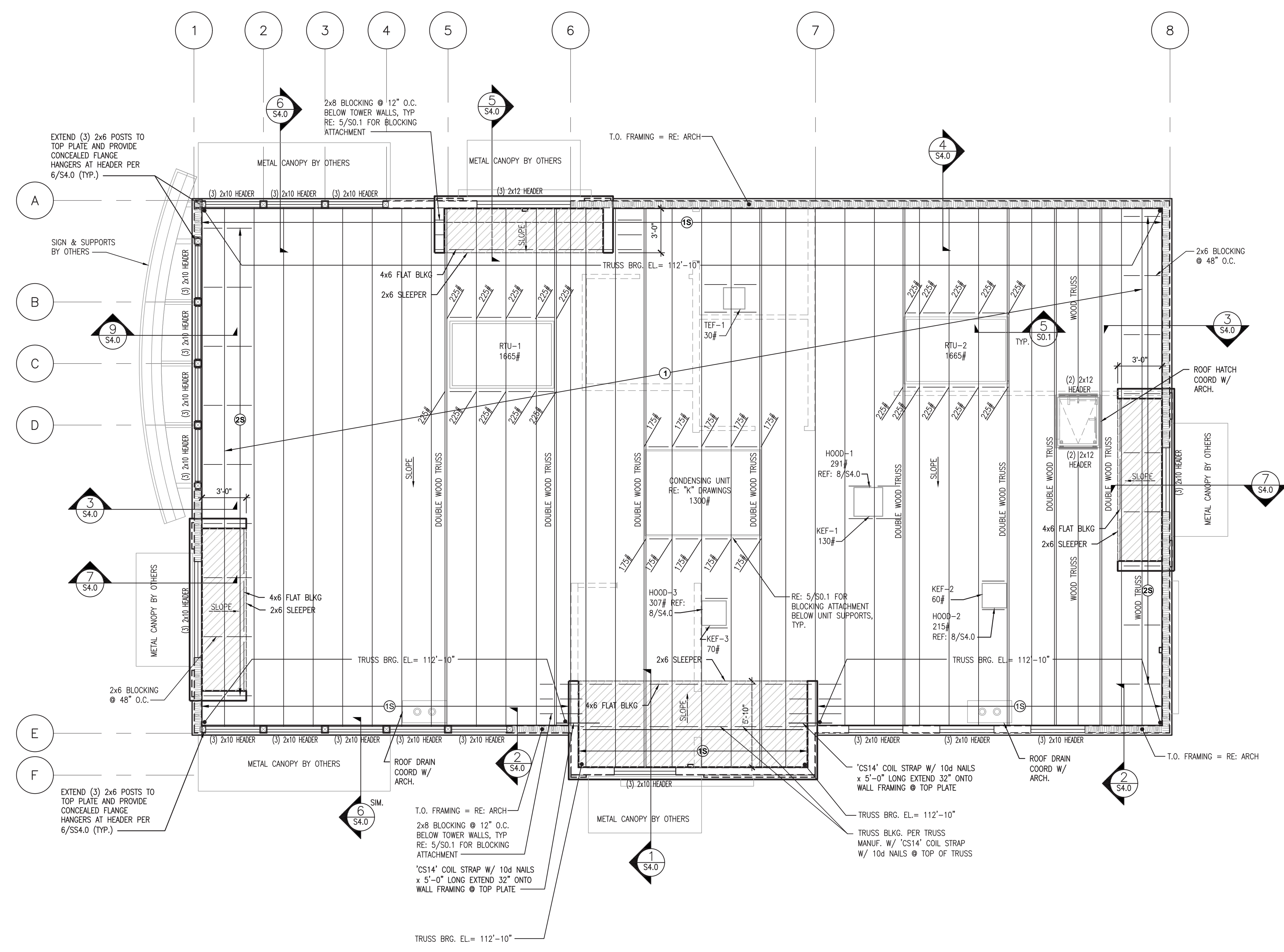
- INDICATES PRE-MANUF WOOD ROOF TRUSS @ 24" O.C. MAX. DOUBLE WOOD TRUSSES TO BE LOCATED @ RTU'S AND ROOF HATCH OPNG AS SHOWN.

**ROOF TRUSS: SNOW DRIFT:**

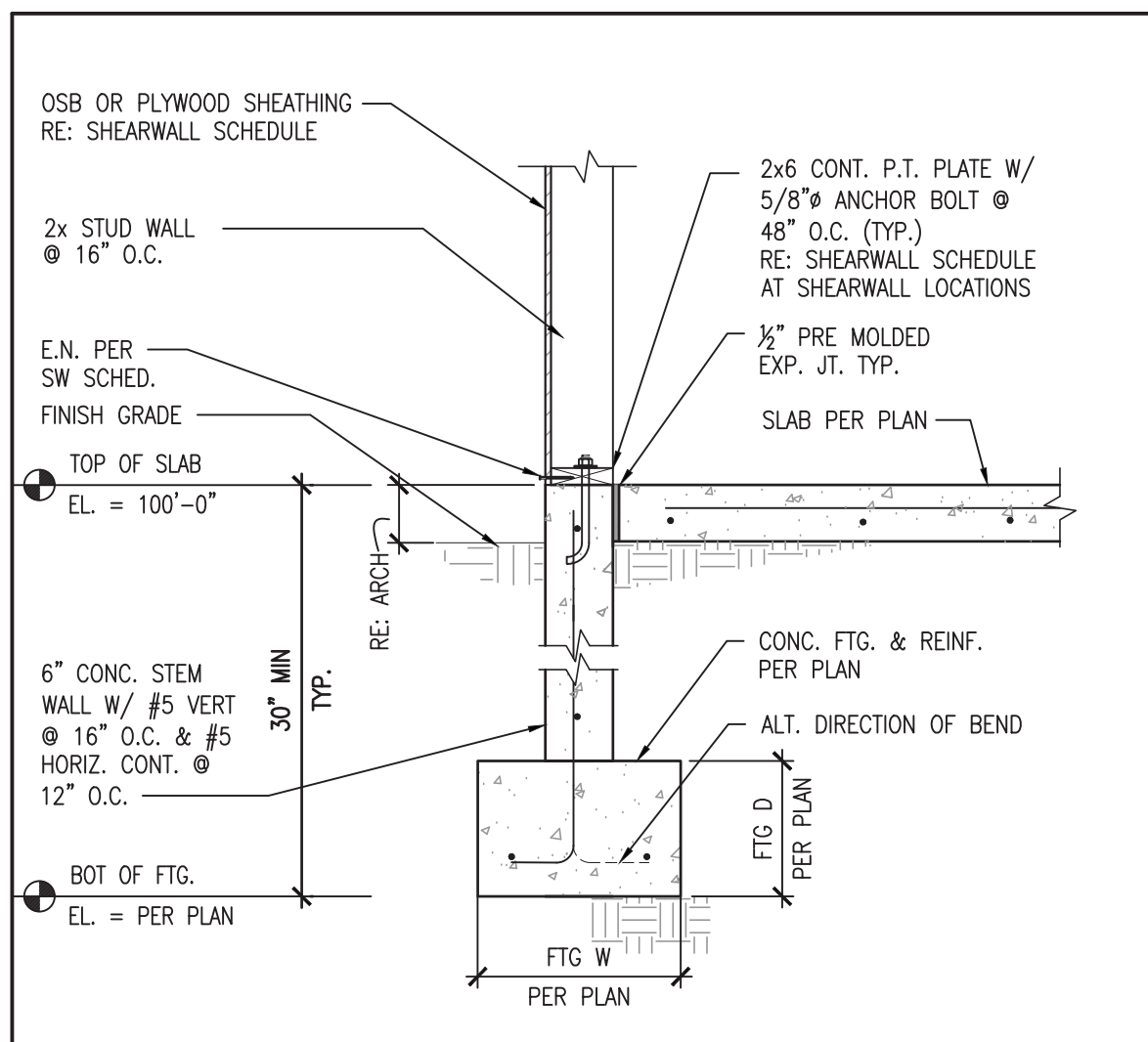


SNOW DRIFT SCHEDULE			
MARK	(A) (PSF)	(B) (PSF)	W (FT)
1S	34	25	7'-3"
2S	44	25	9'-3"

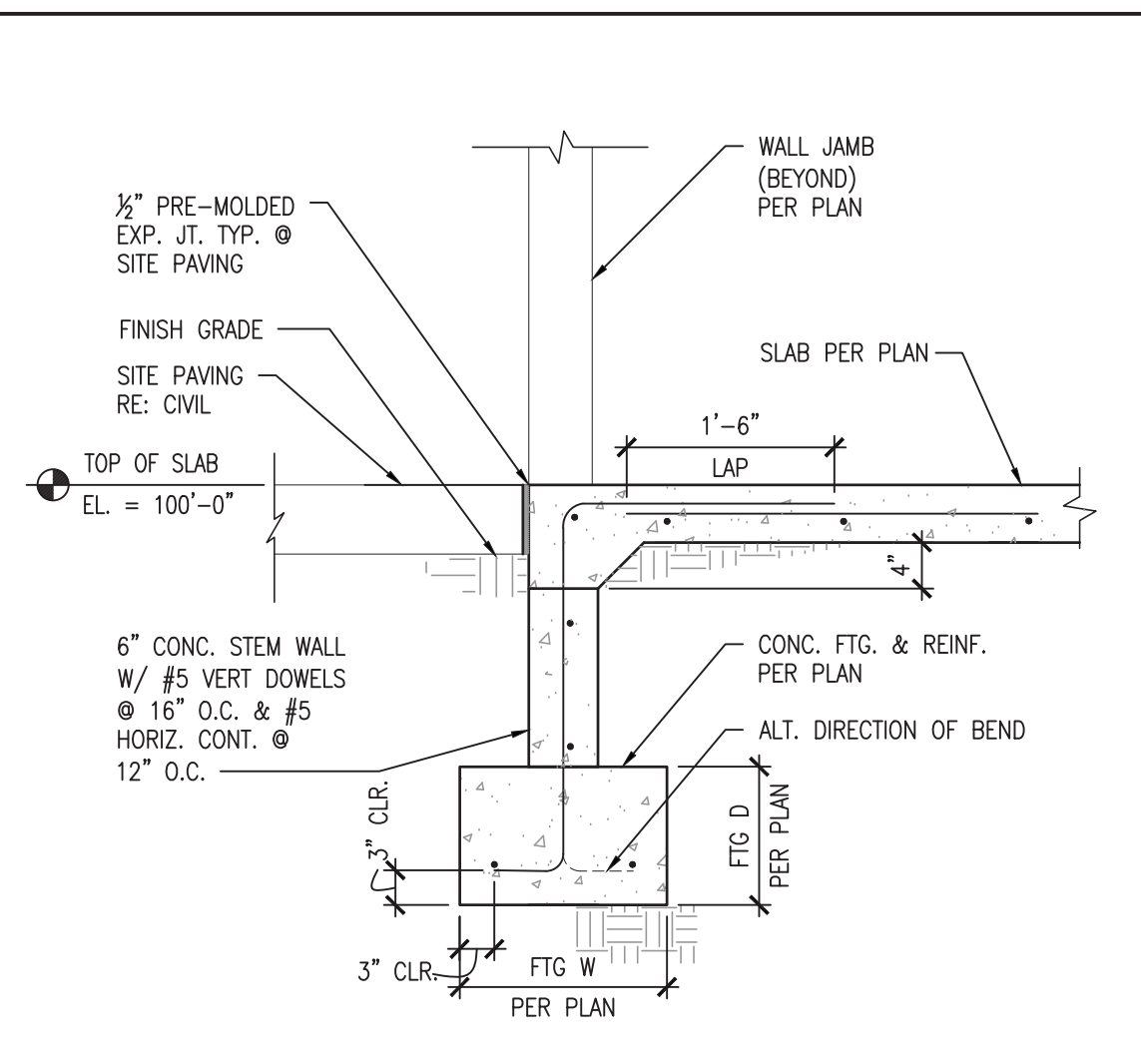
- NOTES:**
- (A) = THE MAXIMUM TRIANGULAR SURCHARGE LOAD DUE TO DRIFTING AND IS TO BE ADDED TO THE B LOAD. (A) = 0 PSF AT THE BOTTOM OF THE TRIANGLE LOAD AT A DISTANCE W).
  - (B) = THE UNIFORM SNOW LOAD.
  - UNIFORM SNOW LOAD = 25 PSF WHEN SNOW DRIFTS ARE APPLIED.
  - SNOW DRIFT @ MECHANICAL EQUIPMENT LOCATIONS: (1S) @ ALL (4) SIDES OF EQUIPMENT



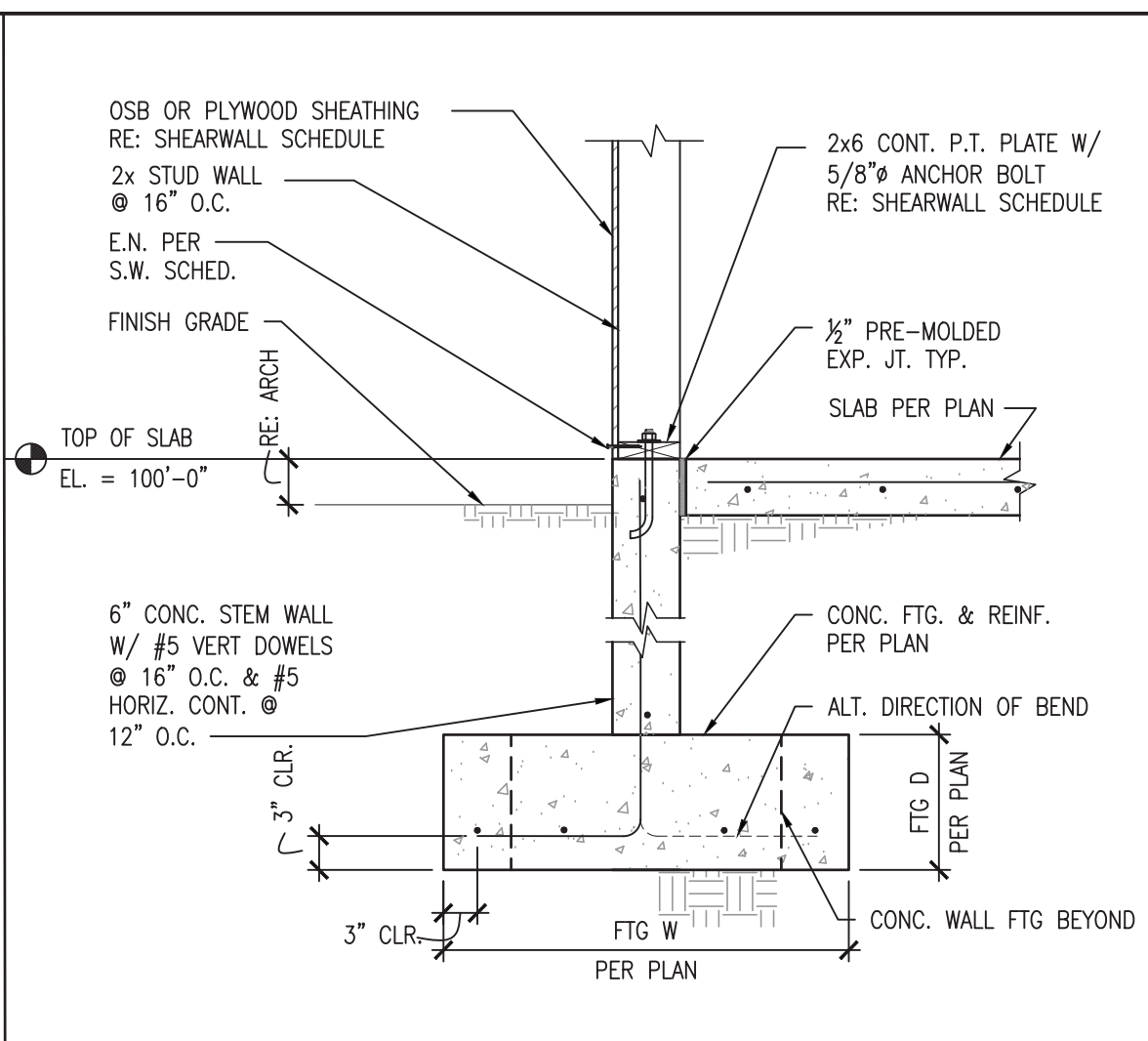




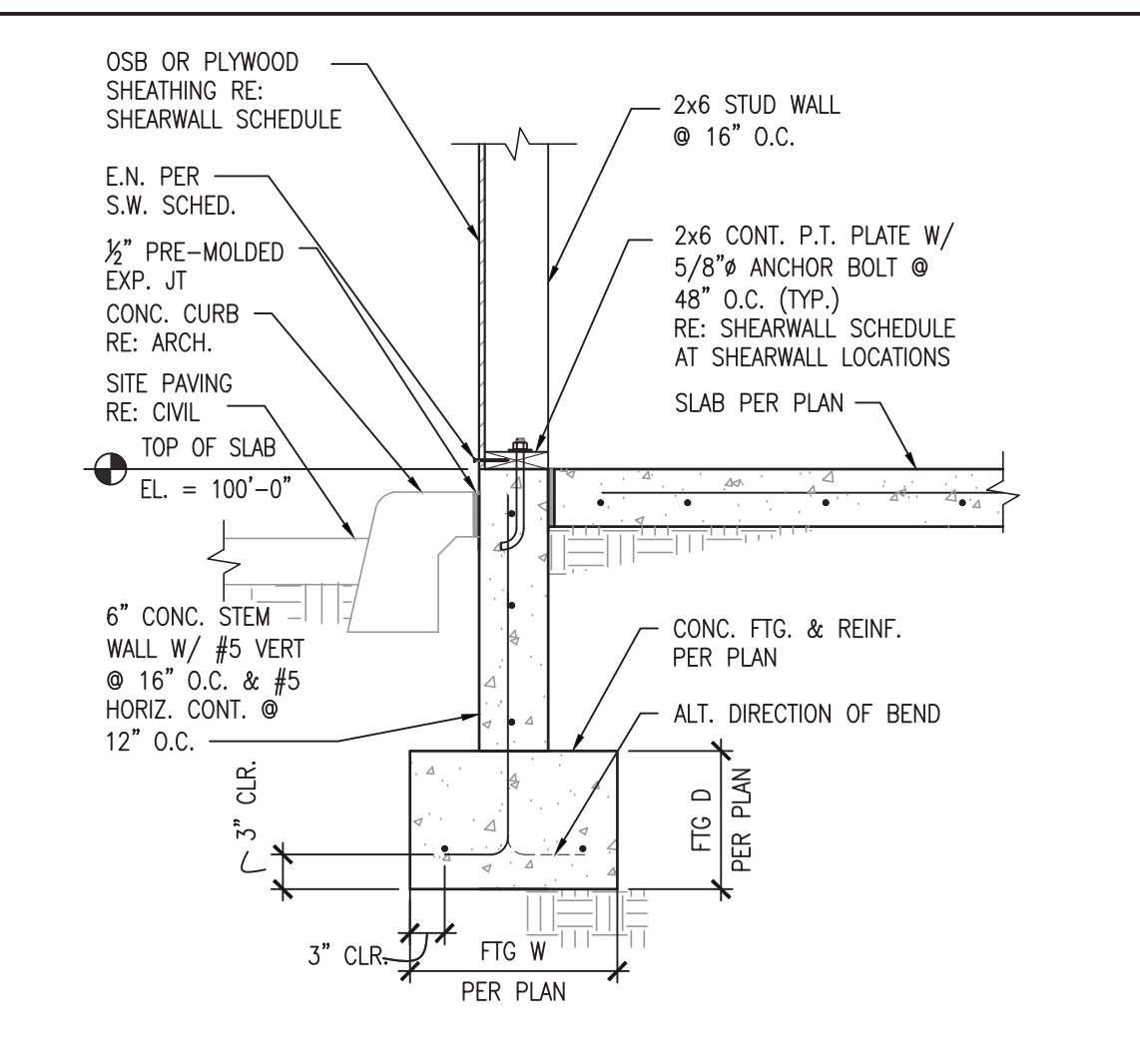
TYPICAL PERIMETER FOOTING  
3/4"=1'-0" 1



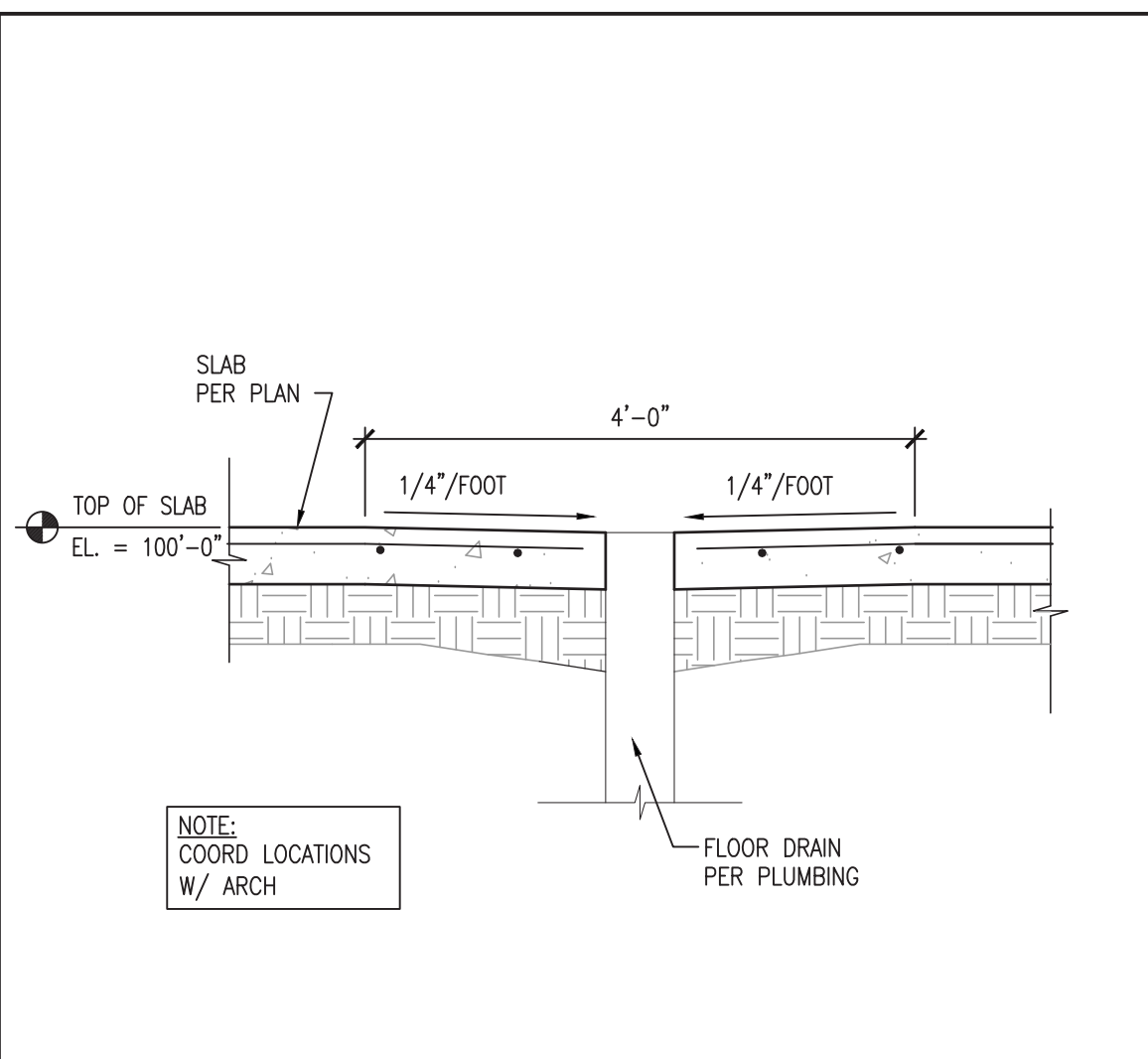
TYPICAL PERIMETER FOOTING AT DOOR DETAIL  
3/4"=1'-0" 2



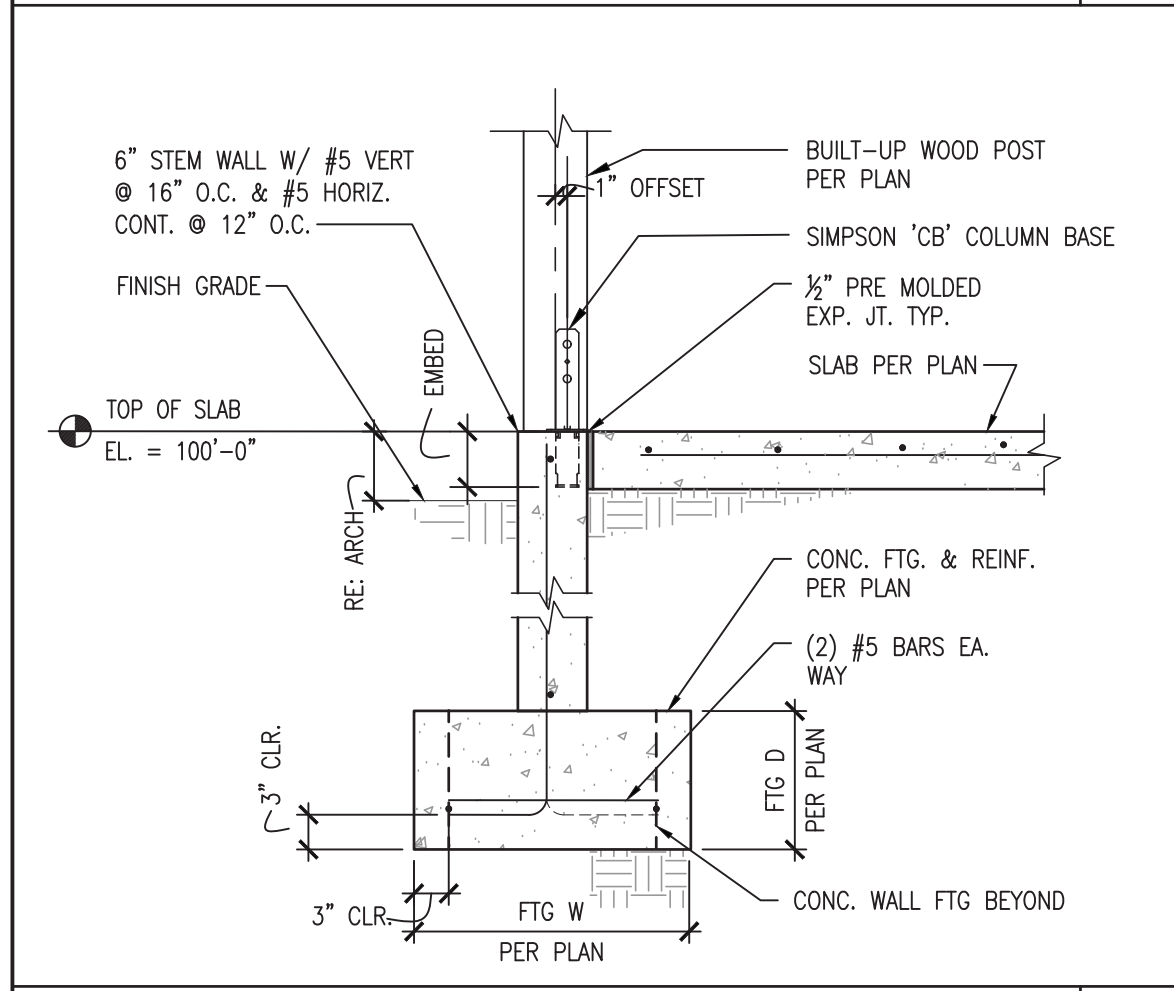
EXTERIOR SHEAR WALL FOOTING  
3/4"=1'-0" 3



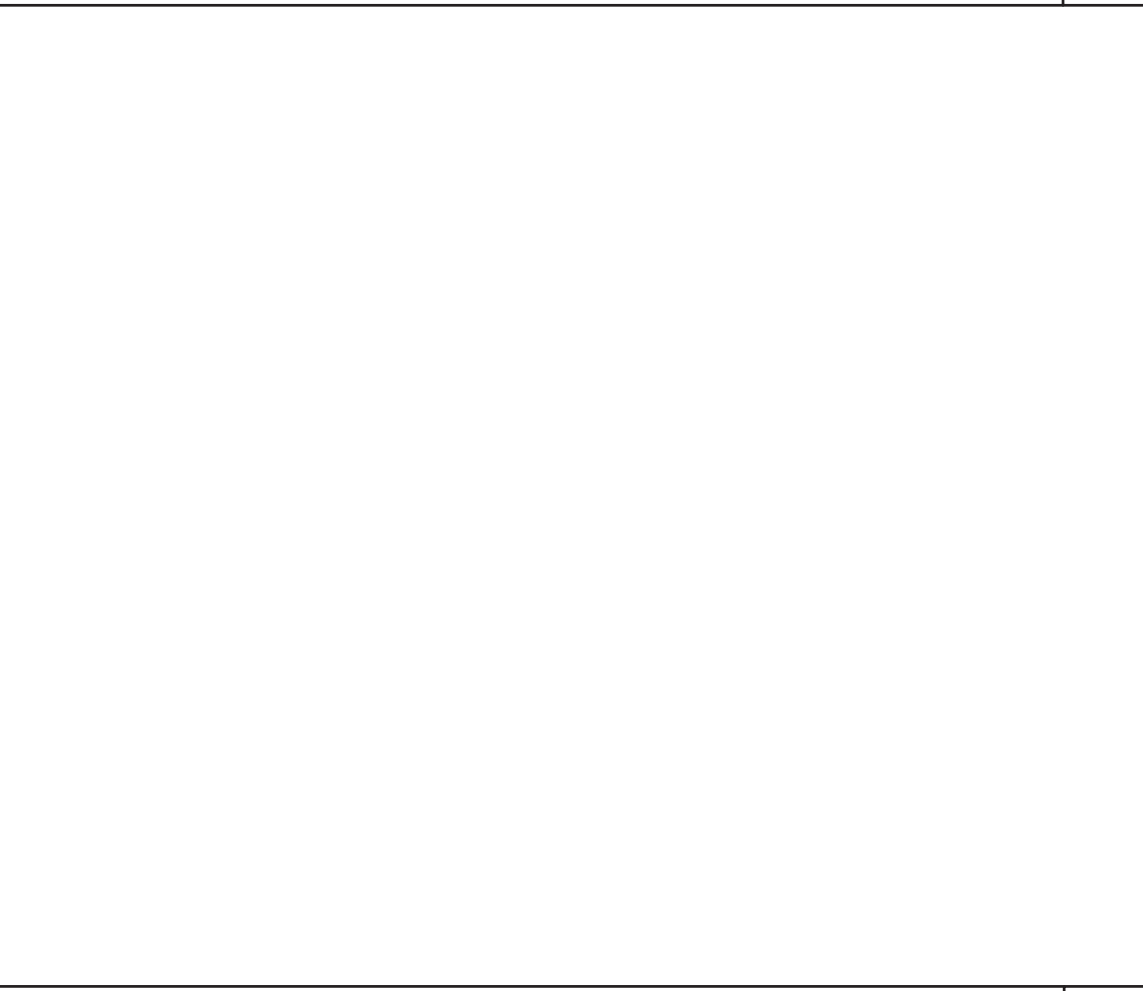
FOOTING AT DRIVE-THRU TOWER DETAIL  
3/4"=1'-0" 4



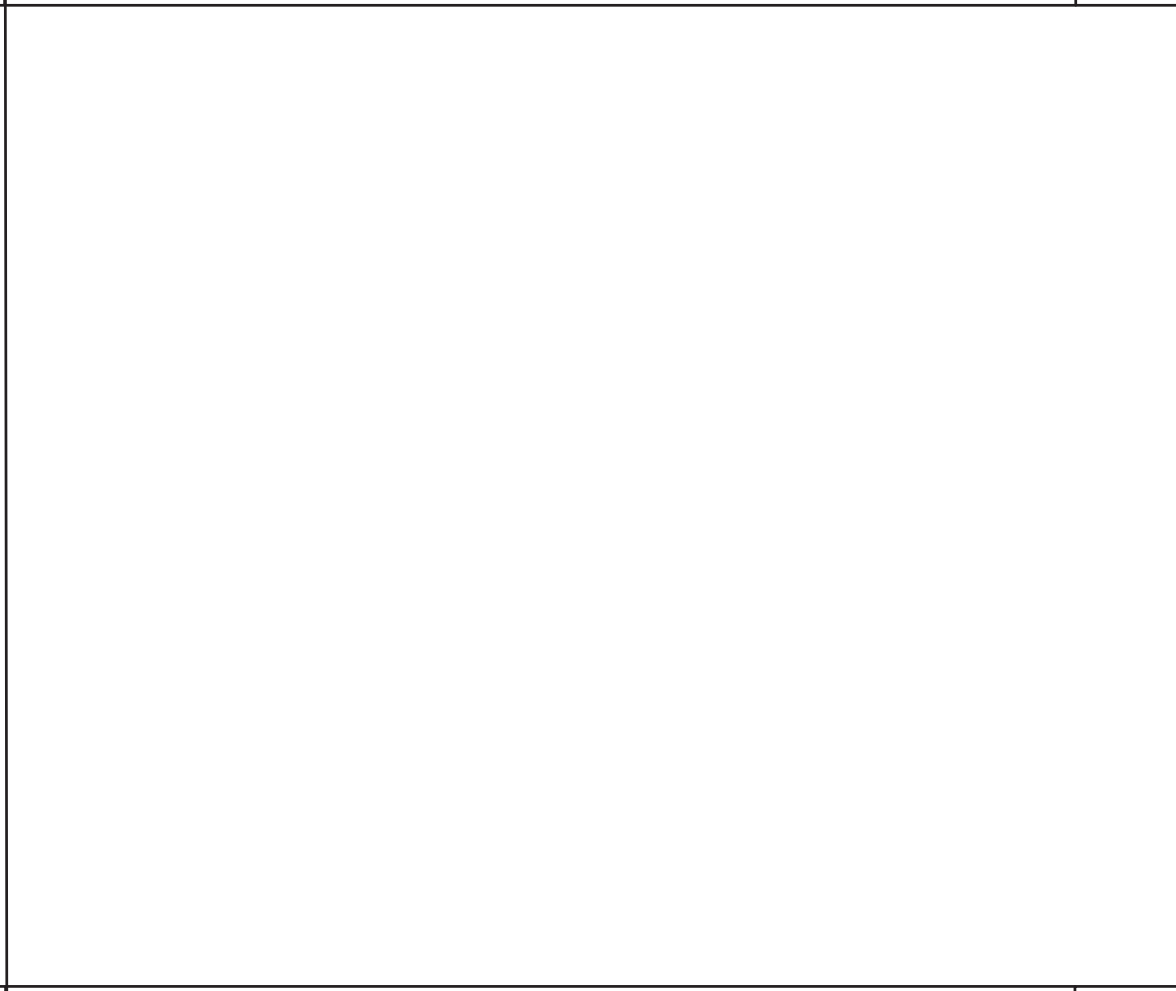
TYPICAL SLOPED FLOOR DRAIN DETAIL  
3/4"=1'-0" 5



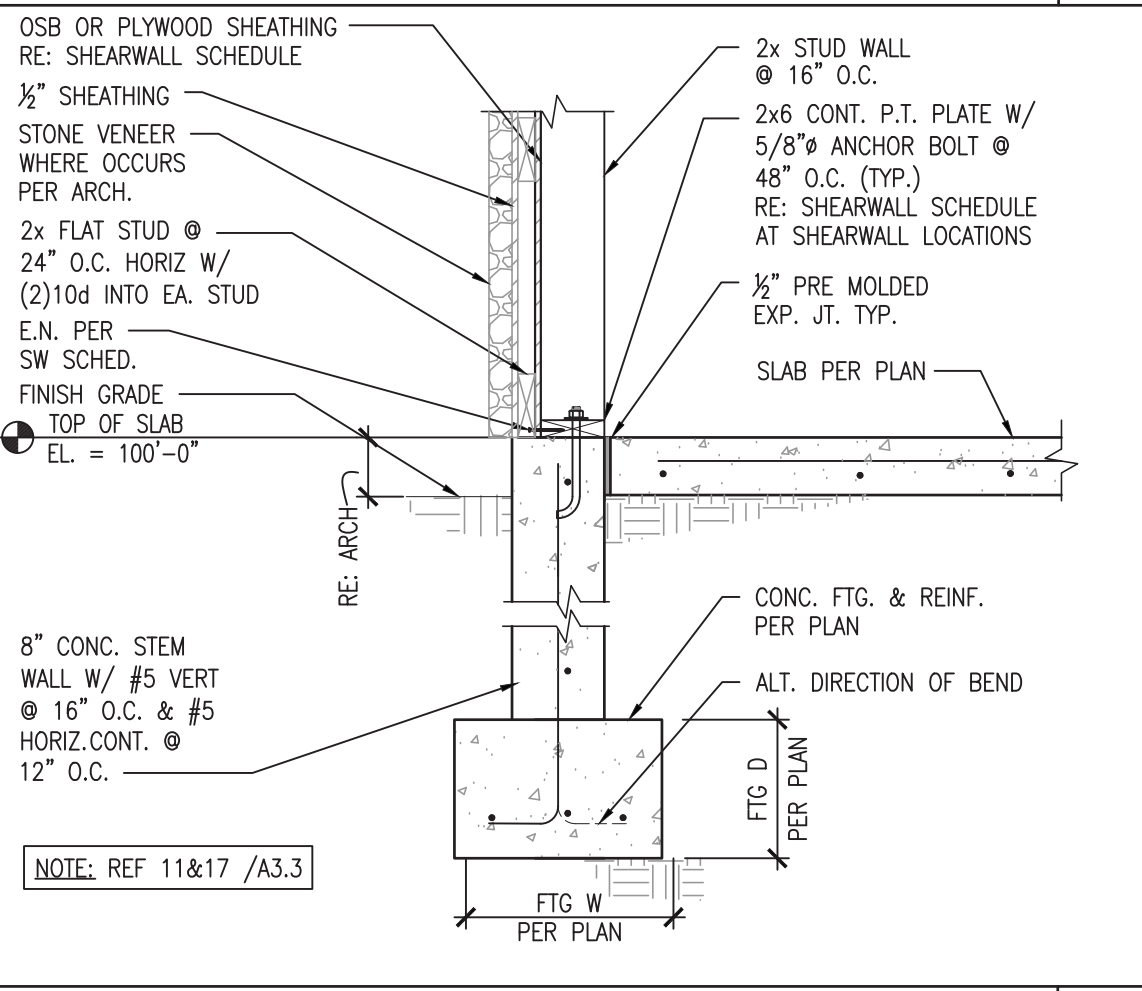
TYPICAL POST TO FOOTING CONN.  
3/4"=1'-0" 6



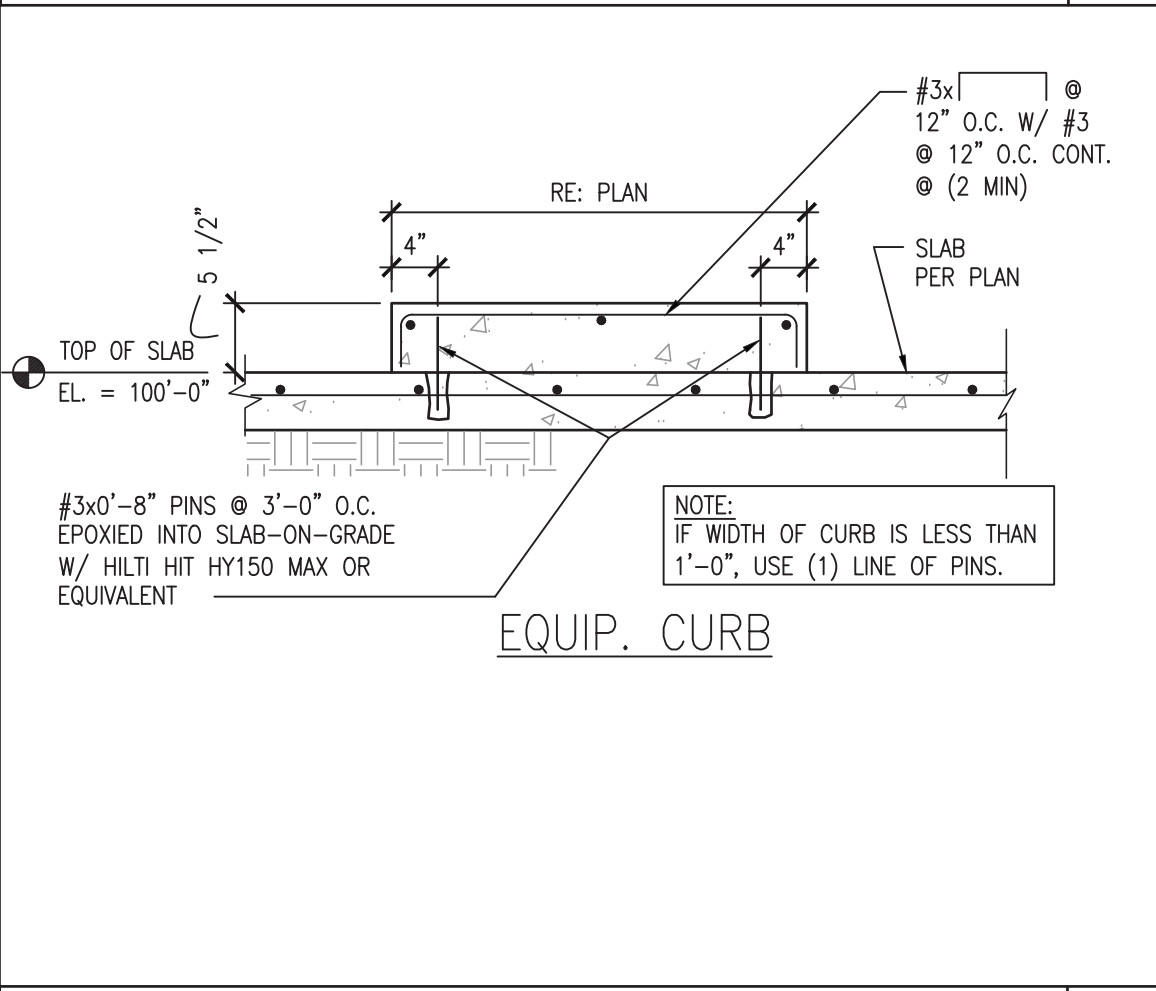
NOT USED 7



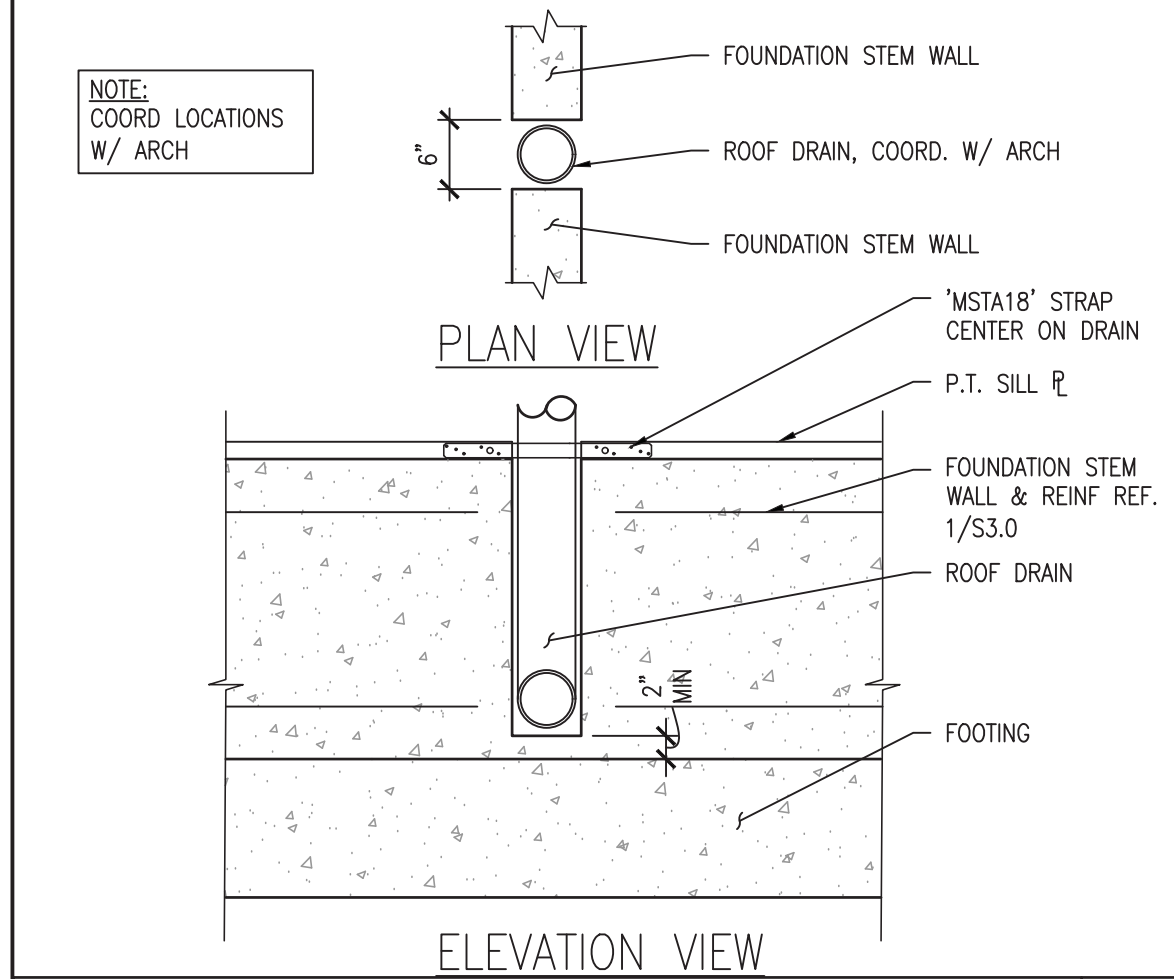
NOT USED 8



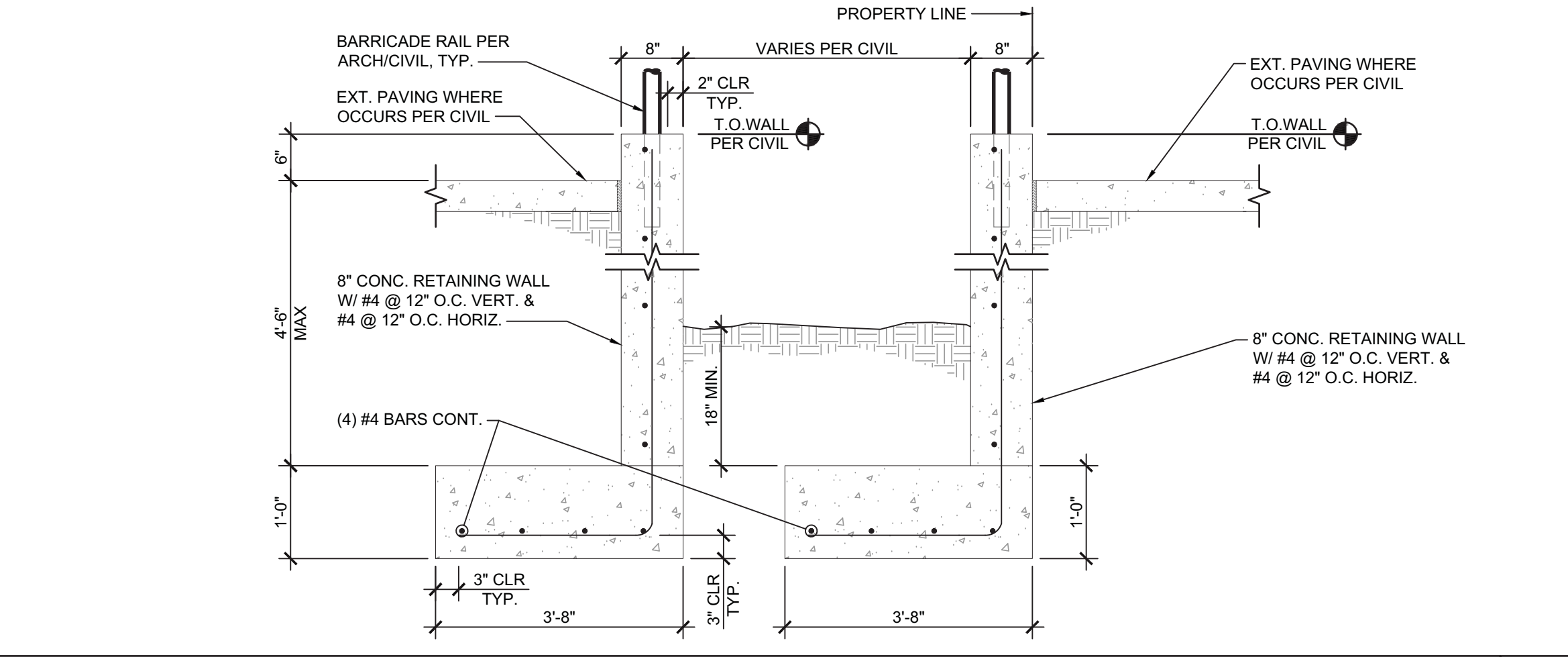
PERIMETER FOOTING @ WALL BUMP OUT  
3/4"=1'-0" 9



TYPICAL CONCRETE CURB DETAIL  
3/4"=1'-0" 10



ROOF DRAIN @ FOUNDATION WALL  
3/4"=1'-0" 11



CONC. RETAINING WALL & FTG @ RUNOFF SWALE  
3/4"=1'-0" 12



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310 NORTH 5th Street, Suite 83702  
Spokane, WA 99201  
(509) 331-9031  
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DRAWN: KM  
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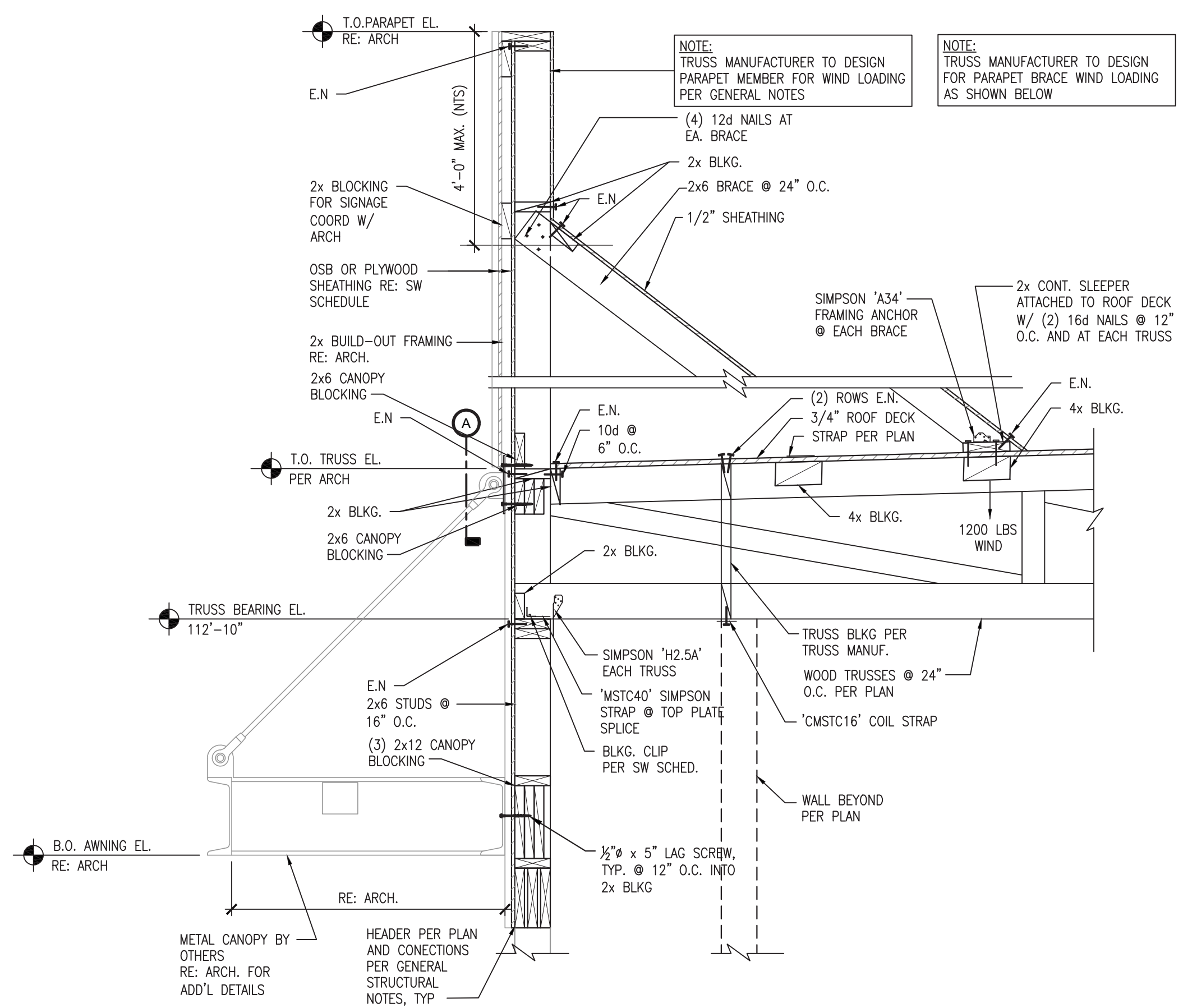
DATE	DESCRIPTION

SHEET TITLE:  
**FOUNDATION DETAILS**

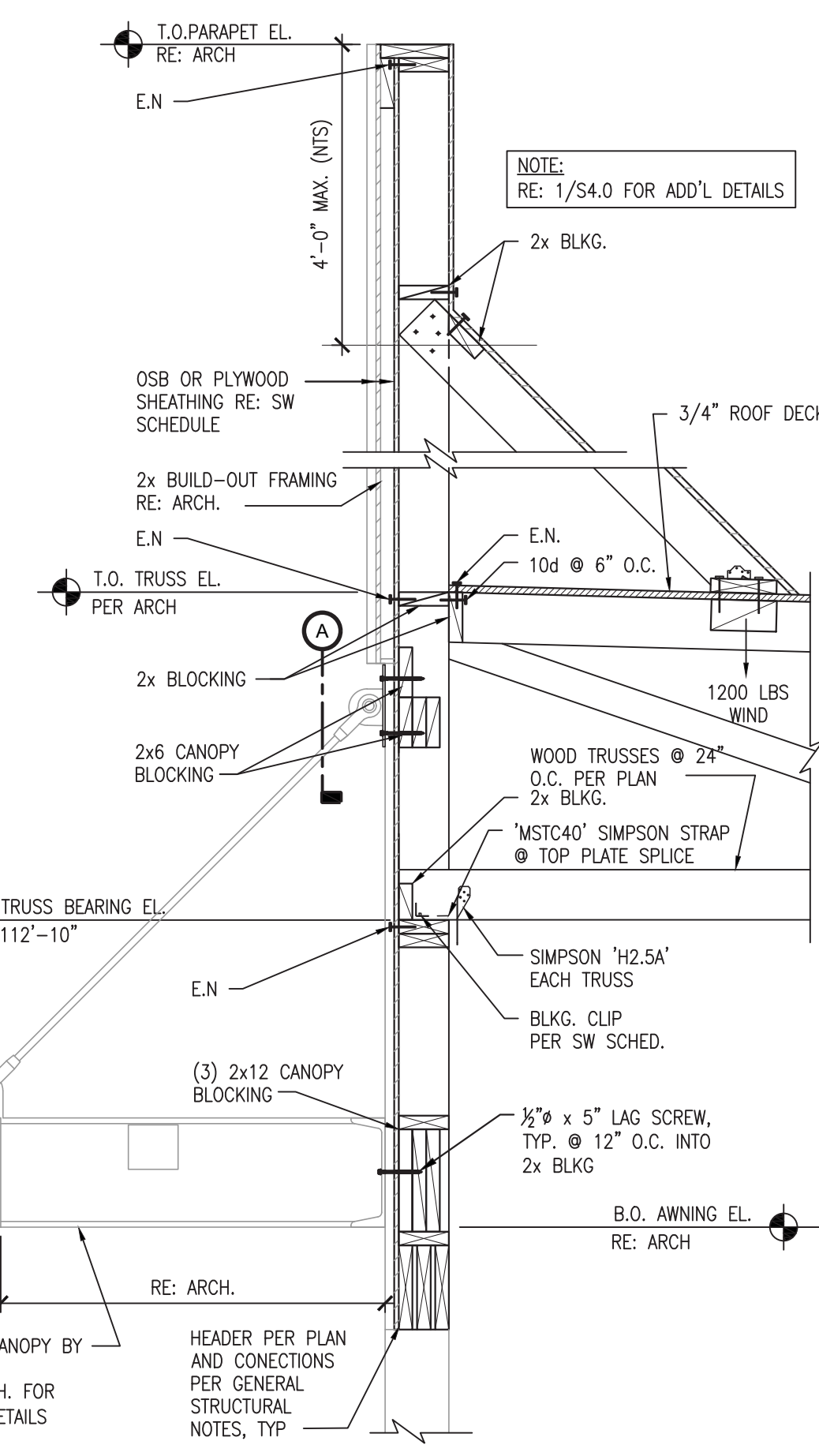
SHEET NUMBER:

**S3.0**

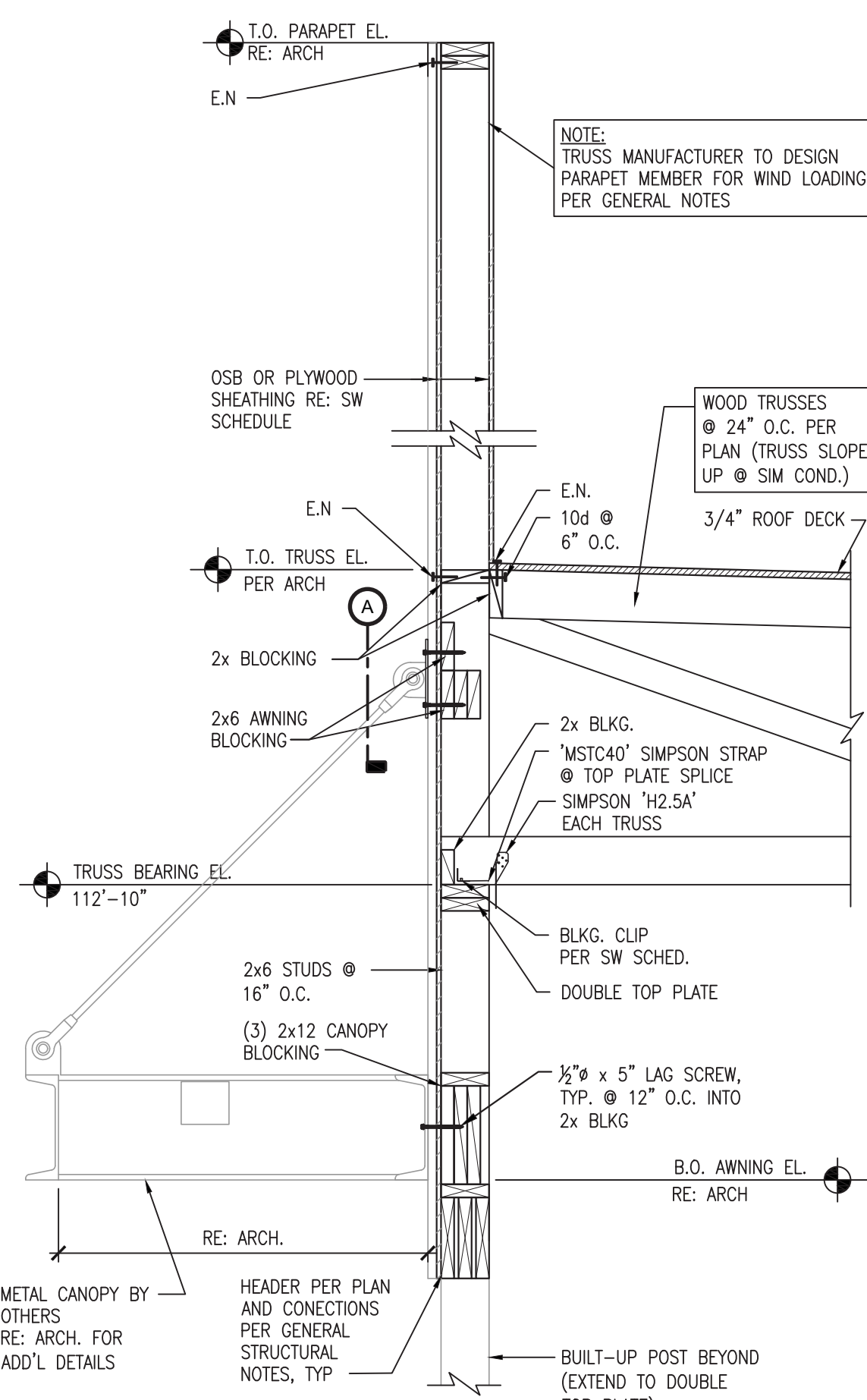




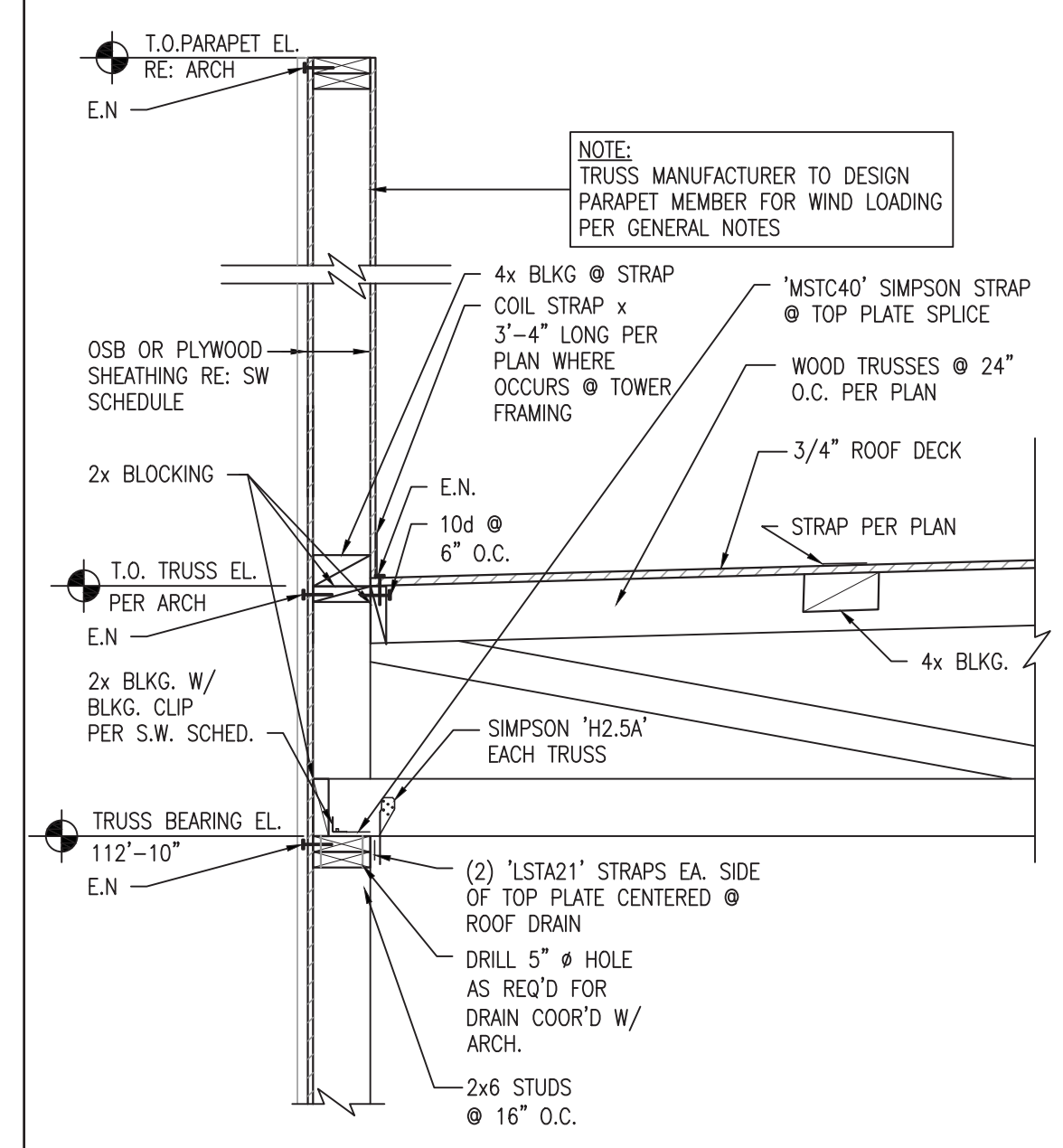
**ROOF LOW SIDE @ WALL AT TOWER PARAPET DETAIL**  
3/4"=1'-0" **1**



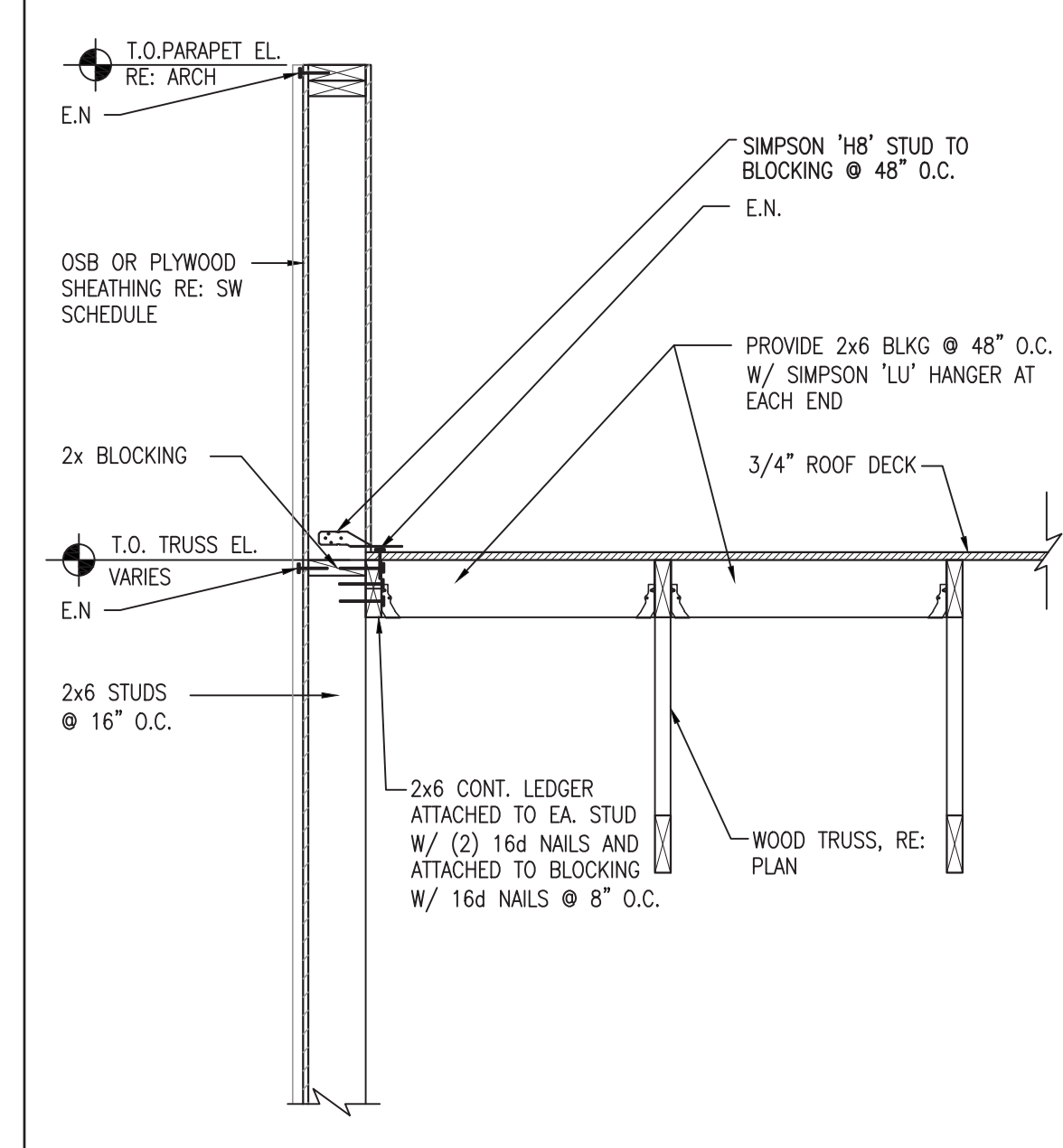
**ROOF HIGH SIDE @ WALL AT TOWER PARAPET DETAIL**  
3/4"=1'-0" **5**



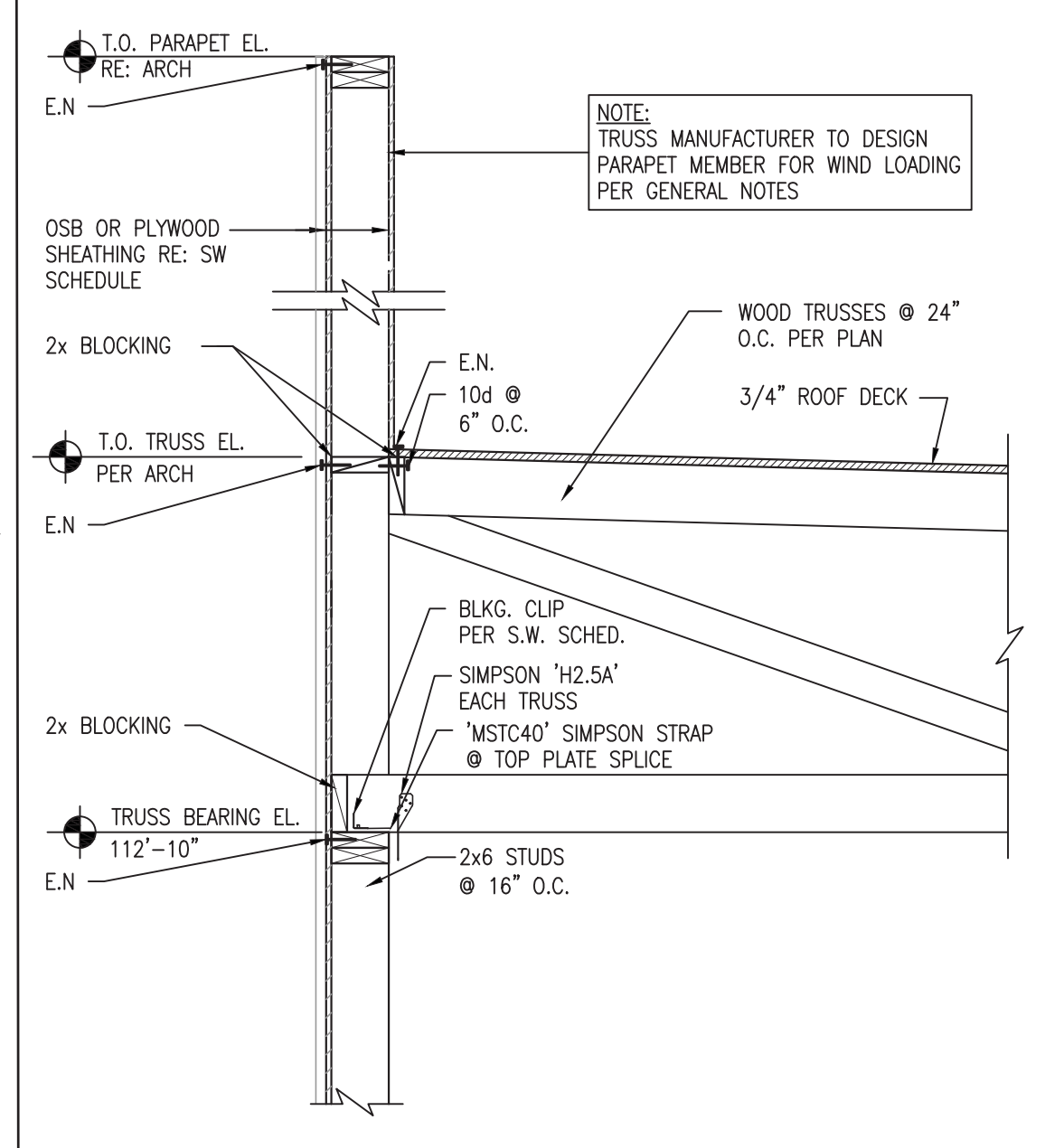
**ROOF HIGH SIDE @ WALL AT METAL CANOPY DETAIL**  
3/4"=1'-0" **6**



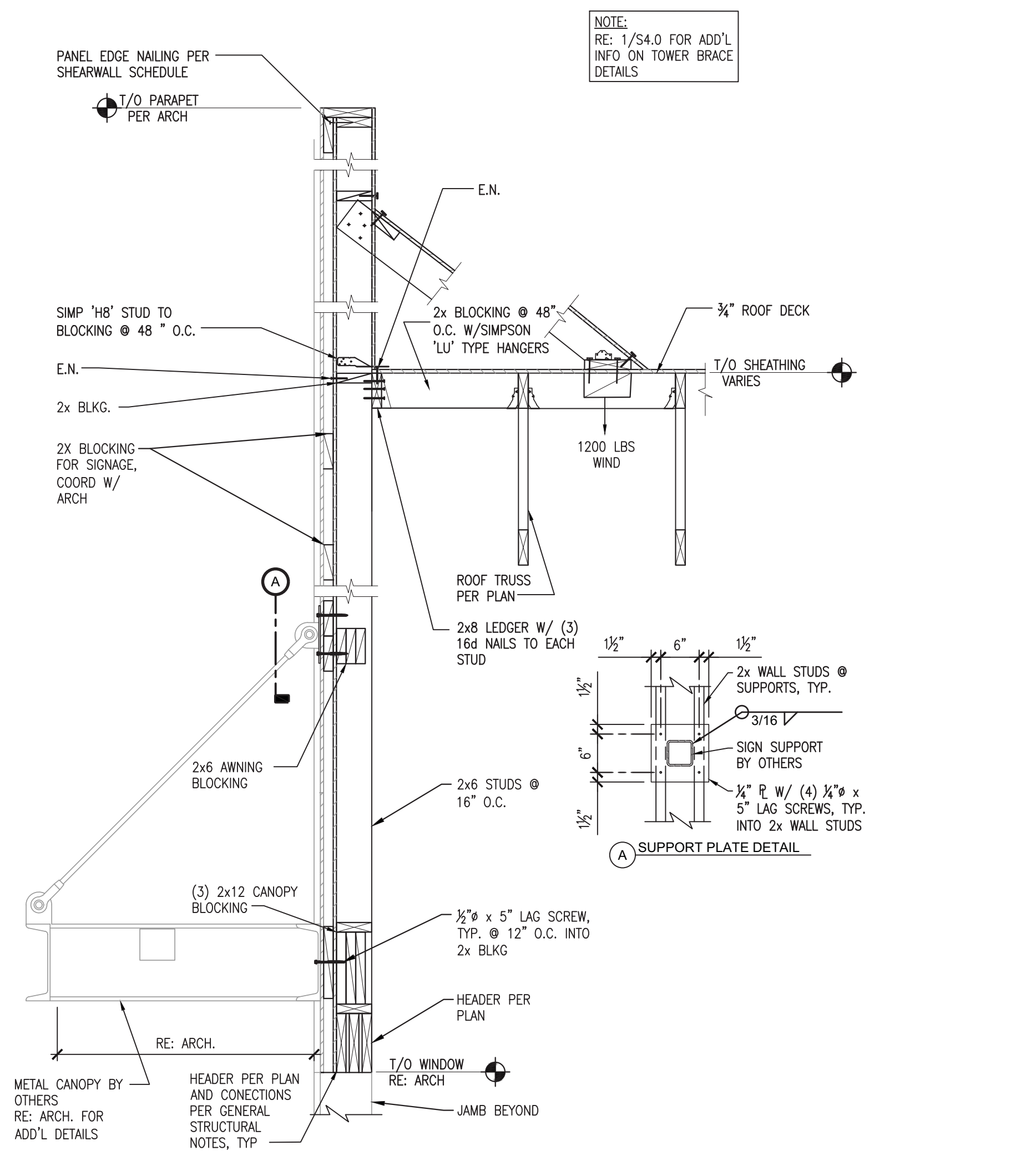
**ROOF LOW SIDE @ WALL DETAIL**  
3/4"=1'-0" **2**



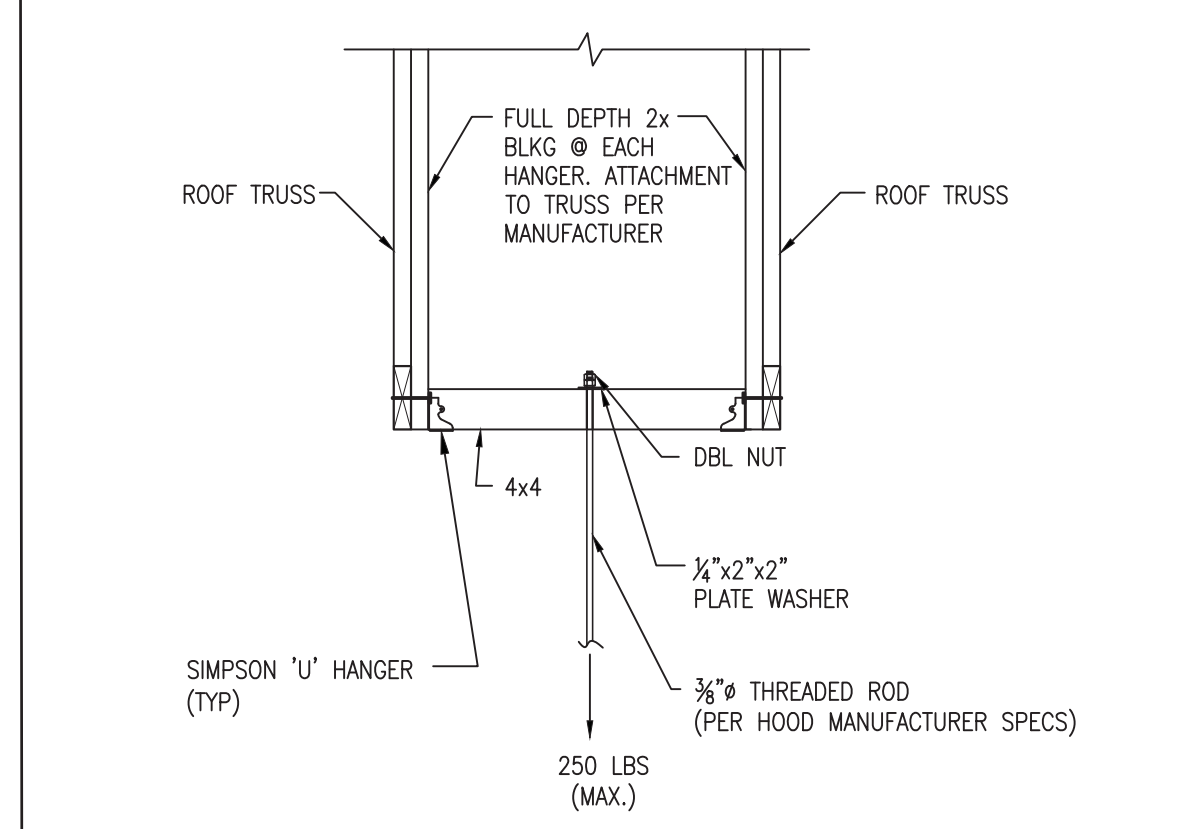
**SIDE WALL DETAIL**  
3/4"=1'-0" **3**



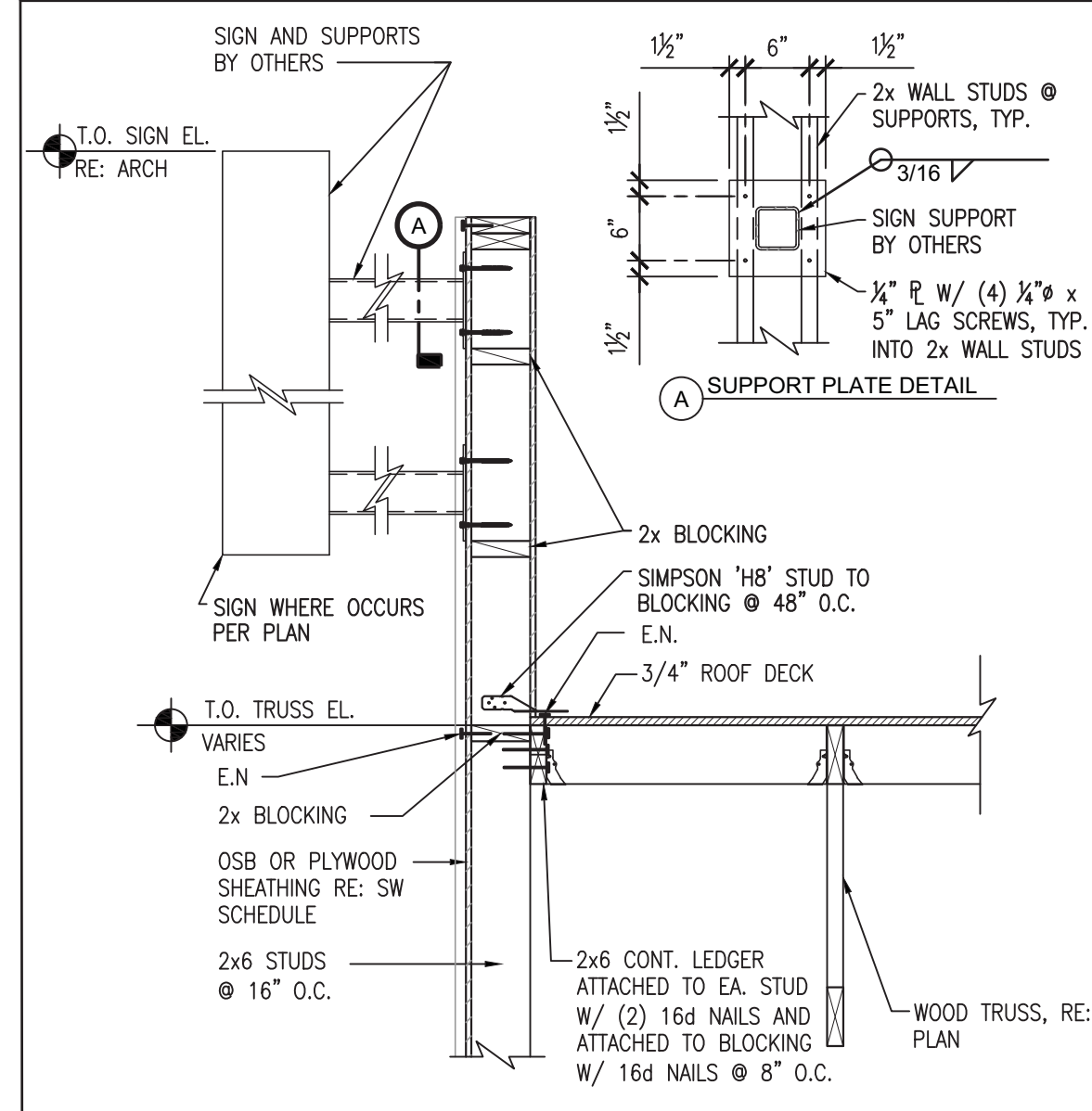
**ROOF HIGH SIDE @ WALL DETAIL**  
3/4"=1'-0" **4**



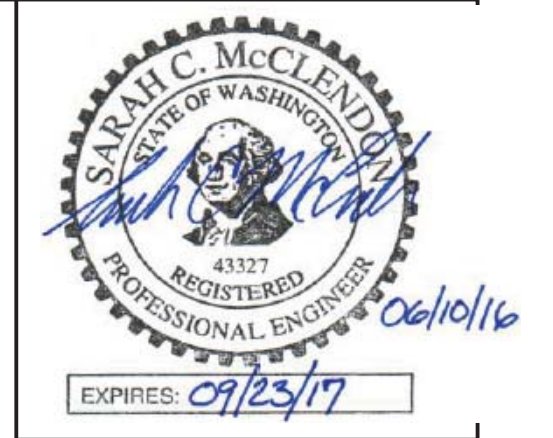
**CANOPY SUPPORT - PARALLEL ROOF TRUSS**  
3/4"=1'-0" **7**



**TYPICAL HANGER ROD DETAIL**  
3/4"=1'-0" **8**



**SIGN ATTACHMENT @ SIDE WALL**  
3/4"=1'-0" **9**



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310 NORTH 5th Street, Suite 83702  
Spokane, WA 99201  
(509) 331-9031  
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DATE	DESCRIPTION

**ROOF FRAMING DETAILS**

SHEET NUMBER:

**S4.0**